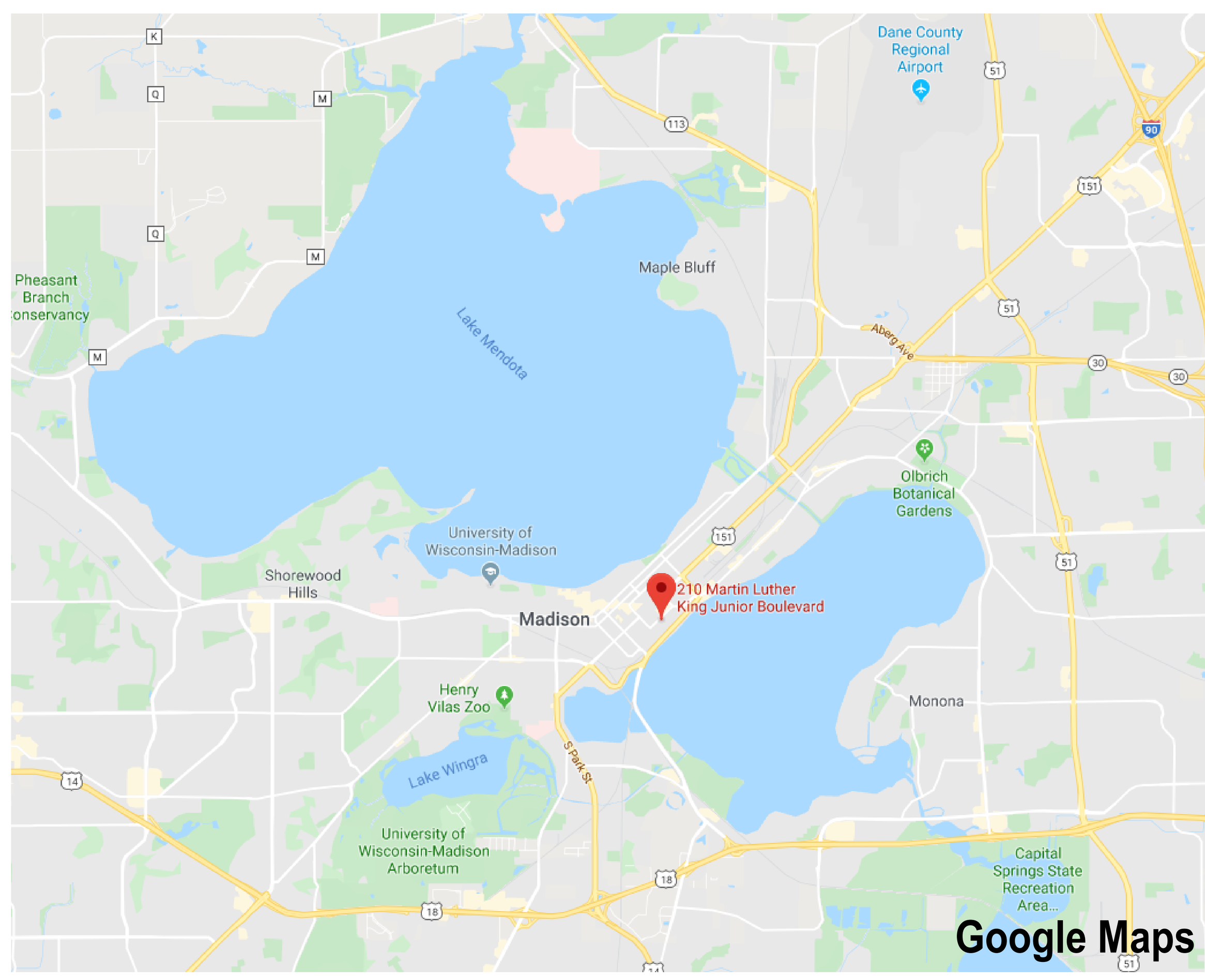
NEW REMOTE DESCENT SYSTEM ANCHORAGE

CITY-COUNTY BUILDING
MADISON, WI

CONSTRUCTION DOCUMENTS
DECEMBER 30, 2019
COUNTY OF DANE



SHEET LIST	
SHEET NO	SHEET NAME
S-001	GENERAL NOTES
S-002	ABBREVIATIONS
S-161	SIXTH FLOOR PARTIAL PLAN - WEST
S-181	MAIN ROOF PLAN - EAST
S-182	MAIN ROOF PLAN - WEST
S-191	PENTHOUSE ROOF PLAN - EAST
S-192	PENTHOUSE ROOF PLAN - WEST
S-301	STRUCTURAL DETAILS
S-401	DEMOLITION AND WATERPROOFING DETAILS

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PROFESSIONAL ENGINEER
12/30/2019
Exp. 7.31.2020

REV.	ISSUE	DATE
2	CONSTRUCTION DOCUMENTS	12/30/19

NEW REMOTE DESCENT SYSTEM ANCHORAGE

CITY-COUNTY BUILDING
210 MARTIN LUTHER KING JR BLVD
MADISON, WI 53703

TITLE
COVER SHEET & DRAWING LIST

PROJECT NO: C19150.00
SCALE:
DRAWN BY: JRC
CHECKED BY: RTM
DATE: 11/05/2019

SHEET NUMBER
S-000

GR GENERAL REQUIREMENTS

GR-1 AS USED IN THESE GENERAL NOTES:
"DRAWINGS" MEANS THE LATEST PROJECT DESIGN DRAWINGS, UON.
"SPECIFICATIONS" MEANS THE LATEST PROJECT SPECIFICATIONS, UON.
"CONTRACT DOCUMENTS" IS DEFINED AS THE DESIGN DRAWINGS AND THE SPECIFICATIONS
"SER" IS DEFINED ENGINEER OF RECORD FOR THE STRUCTURE IN ITS FINAL CONDITION (I.E. THORNTON TOMASETTI).
"MEP" INCLUDES, BUT IS NOT LIMITED TO MECHANICAL, ELECTRICAL, PLUMBING, FIRE PROTECTION.
"CONTRACTOR" IS DEFINED TO INCLUDE ANY OF THE FOLLOWING: GENERAL CONTRACTOR AND THEIR SUBCONTRACTORS, CONSTRUCTION MANAGER AND THEIR SUBCONTRACTORS, STRUCTURAL STEEL FABRICATOR OR STRUCTURAL STEEL ERECTOR.
"BASE BUILDING STRUCTURE" IS DEFINED AS THE STRUCTURAL FRAME DESIGNED BY THORNTON TOMASETTI.
"STRUCTURE IN ITS FINAL CONDITION" MEANS ALL STRUCTURAL ELEMENTS SHOWN ON THE STRUCTURAL CONTRACT DOCUMENTS ARE INSTALLED AND COMPLETELY CONNECTED AND INSPECTED WITH NO OUTSTANDING NON-COMPLIANCE ISSUES.
"DELEGATED DESIGN" MEANS A SCOPE OF WORK THAT MEETS PERFORMANCE CRITERIA ESTABLISHED IN THE CONTRACT DOCUMENTS AND IS TO BE COMPLETED BY THE CONTRACTOR'S LICENSED ENGINEER.

GR-2 THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN, INSTALLATION, AND REMOVAL OF TEMPORARY BRACING AND CONSTRUCTION SUPPORTS, FOR NEW AND EXISTING STRUCTURES, AS NECESSARY TO COMPLETE THE PROJECT. NO PORTION OF THE PROJECT WHILE UNDER CONSTRUCTION IS INTENDED TO BE STABLE IN THE ABSENCE OF THE CONTRACTOR'S TEMPORARY SUPPORTS AND BRACES. CONTRACTOR SHALL RETAIN A STRUCTURAL ENGINEER LICENSED IN THE STATE OF WISCONSIN TO DESIGN TEMPORARY BRACING AND CONSTRUCTION SUPPORTS.

GR-3 THE SPECIFICATIONS ARE AN INTEGRAL PART OF THE CONTRACT DOCUMENTS AND SHALL BE USED IN CONJUNCTION WITH THE STRUCTURAL DRAWINGS.

GR-4 THE CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS AND COORDINATE WITH THE CONTRACT DOCUMENTS, PROJECT SHOP DRAWINGS AND FIELD CONDITIONS.

GR-5 IN CASES OF CONFLICT BETWEEN DRAWINGS AND/OR SPECIFICATIONS OR EXISTING CONDITIONS, CONTRACTOR SHALL NOTIFY THORNTON TOMASETTI AND OBTAIN CLARIFICATION PRIOR TO BIDDING AND PROCEEDING WITH WORK.

GR-6 APPLY DETAILS, SECTIONS, AND NOTES ON THE DRAWINGS WHERE CONDITIONS ARE SIMILAR TO THOSE INDICATED BY DETAIL, DETAIL TITLE OR NOTE.

GR-7 ONLY USE DIMENSIONS INDICATED ON THE DRAWINGS. DO NOT SCALE DRAWINGS.

GR-8 ASSUME EQUAL SPACING BETWEEN ESTABLISHED DIMENSIONS, IF NOT INDICATED ON DRAWINGS.

GR-9 CENTERLINES OF COLUMNS AND FOUNDATIONS COINCIDE WITH GRID LINE INTERSECTIONS, UON.

GR-10 CENTERLINES OF FRAMING MEMBERS COINCIDE WITH COLUMN CENTERLINES, UON.

GR-11 THE CONTRACTOR SHALL PROTECT EXISTING FACILITIES, STRUCTURES AND UTILITIES FROM DAMAGE.

GR-12 THE CONTRACTOR SHALL VERIFY THAT CONSTRUCTION LOADS DO NOT EXCEED THE CAPACITY OF THE STRUCTURE AT THE TIME THE LOAD IS APPLIED.

CD CODES AND DESIGN CRITERIA

CD-1 PERFORM ALL CONSTRUCTION IN CONFORMANCE WITH THE BUILDING AND DESIGN CODES REFERENCED WITHIN THESE DOCUMENTS. THE PROJECT DOCUMENTS REFER TO THE FOLLOWING CODES AND STANDARDS, UON:

WISCONSIN COMMERICAL BUILDING CODE, 2018
INTERNATIONAL EXISTING BUILDING CODE, 2015
OSHA STANDARDS - PART 1910

STRUCTURAL CONCRETE:
"BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
THE AMERICAN CONCRETE INSTITUTE (ACI 318-14)

STRUCTURAL STEEL:
"SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS", (AISC 360-10)
CONFORMING TO THE PROVISIONS OF LOAD RESISTANCE FACTOR DESIGN,
BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC-LRFD)

CD-2 LIVE LOADS:

ROOFS	20 LBS./SQ. FT.

CD-3 SUPERIMPOSED DEAD LOADS:

ROOFING	5 LBS./SQ. FT.
LWC TOPPING (WHERE OCCURS)	30 LBS./SQ. FT.
MEP BELOW	10 LBS./SQ. FT.
CEILING BELOW	5 LBS./SQ. FT.

CD-4 RISK CATEGORY III

CD-5 WIND LOAD DESIGN DATA:

MAIN WIND FORCE RESISTING SYSTEM	
BASIC WIND SPEED, V	120 MPH
EXPOSURE	D
INTERNAL PRESSURE COEFFICIENT	± 0.18

COMPONENT AND CLADDING DESIGN PRESSURES
ROOF EFFECTIVE WIND AREA = 10 SF
ROOF= +/- 73 PSF
ROOF EDGE = +/- 119 PSF
ROOF CORNER= +/- 155 PSF

CD-6 SNOW LOADS NOT CONSIDERED TO ACT SIMULTANEOUSLY WITH TIE BACK ANCHOR LOADS.

CD-7 STRUCTURAL COMPONENTS ARE NOT DESIGNED FOR VIBRATING EQUIPMENT. MOUNT VIBRATING EQUIPMENT ON VIBRATION ISOLATORS.

DE DEMOLITION

DE-1 THE CONTRACTOR IS FULLY RESPONSIBLE FOR THE MEANS AND METHODS OF DEMOLITION AND THE INTEGRITY AND STABILITY OF THE EXISTING STRUCTURE DURING DEMOLITION UNTIL THE WORK IS COMPLETED. THE CONTRACTOR SHALL PROVIDE SHORING IN REQUIRED LOCATIONS WHERE EXISTING CONSTRUCTION TO REMAIN WILL BE AFFECTED BY DEMOLITION. CONTRACTOR SHALL RETAIN A STRUCTURAL ENGINEER LICENSED IN THE STATE OF WISCONSIN TO DESIGN SHORING.

DE-2 THE CONTRACTOR IS RESPONSIBLE FOR REPAIRS TO ANY STRUCTURAL ELEMENTS WHICH ARE TO REMAIN AND THAT HAVE BEEN DAMAGED DURING THE DEMOLITION PROCESS TO THE COMPLETE SATISFACTION OF THE OWNER. THE REPAIRS SHALL BE AT NO EXPENSE TO THE OWNER. ALL REPAIR WORK SHALL BE DESIGNED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE WISCONSIN AND SUBMITTED TO THORNTON TOMASETTI FOR REVIEW AND APPROVAL PRIOR TO COMMENCING REPAIR WORK.

DE-3 ALL EXISTING FRAMING IS INDICATED FOR REFERENCE ONLY AND IS TO BE FIELD VERIFIED BY THE CONTRACTOR. VERIFY THE EXACT EXTENT OF DEMOLITION AT THE SITE. DETERMINE THE NATURE AND EXTENT OF DEMOLITION THAT WILL BE NECESSARY BY COMPARING THE CONTRACT DOCUMENTS WITH THE EXISTING CONSTRUCTION. IMMEDIATELY NOTIFY THE DESIGN PROFESSIONALS OF ANY INCONSISTENCIES.

DE-4 THE CONTRACTOR SHALL USE QUALIFIED, EXPERIENCED PERSONNEL FOR DEMOLITION AND REMOVAL OPERATIONS. PERFORM DEMOLITION AND REMOVAL OPERATIONS IN A CAREFUL AND ORDERLY MANNER TO PREVENT HAZARDS TO PERSONS, DAMAGE TO PROPERTY, AND THE SPREADING OF DUST AND DEBRIS.

DE-5 DO NOT PERMIT PORTIONS OF THE STRUCTURE TO FALL NOR DEBRIS TO DROP EXCEPT BY METHODS WHICH WILL ENSURE INTEGRITY OF THE STRUCTURE.

DE-6 PRIOR TO THE START OF WORK, VERIFY THAT THE SCOPE OF DEMOLITION INDICATED ON THE CONTRACT DOCUMENTS SHALL NOT DAMAGE, CUT OR DISRUPT SERVICE OF ANY MECHANICAL SYSTEM, ELECTRICAL SYSTEM OR UTILITY EMBEDDED IN THE EXISTING STRUCTURE.

DE-7 DO NOT REMOVE MORE OF THE EXISTING STRUCTURE THAN INDICATED ON CONTRACT DOCUMENTS. DO NOT DAMAGE, MAR, CUT OR DEFACE THE REMAINING STRUCTURE OR MATERIALS TO BE REUSED.

DE-8 THE CONTRACTOR SHALL INCLUDE IN HIS BID THE COST OF REMOVING DEMOLISHED MATERIALS FROM THE SITE IN ACCORDANCE WITH ALL APPLICABLE LAWS, CODES, AND REGULATIONS.

DE-9 WHERE NEW OPENINGS IN EXISTING CONCRETE SLABS OR WALLS ARE TO BE CREATED, THE DEMOLITION CONTRACTOR SHALL CORE HOLES AT THE OUTSIDE CORNERS OF THE NEW OPENING PRIOR TO DEMOLITION. SAW-CUT AND DEMOLISH SLAB OR WALL ONLY AFTER THE INSTALLATION OF ALL REQUIRED NEW STRUCTURAL FRAMING AND/OR REINFORCEMENT IN PLAN OR SECTION, UON. SAW CUTTING SHALL BE STRAIGHT AND SHALL NOT EXTEND INTO EXISTING SLAB OR WALL TO REMAIN NOR BEYOND THE HOLES CORED AT THE CORNERS OF THE NEW OPENING.

DI DELEGATED DESIGN ITEMS

DI-1 THE CONTRACTOR SHALL EMPLOY OR RETAIN A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF WISCONSIN TO DESIGN AND DETAIL DELEGATED DESIGN ITEMS TO MEET THE PERFORMANCE AND DESIGN CRITERIA ESTABLISHEDIN THE CONTRACT DOCUMENTS INCLUDING BUT NOT LIMITED TO:

TIE BACK ANCHORS ASSEMBLIES

SU SUBMITTALS

SU-1 TWENTY WORKING DAYS PRIOR TO SUBMITTING SHOP DRAWINGS, THE CONTRACTOR SHALL SUBMIT FOR SER'S REVIEW A SCHEDULE WHICH DETAILS THE ESTIMATED QUANTITY OF SHOP DRAWINGS AND THE DATE THE SHOP DRAWINGS WILL BE RECEIVED BY THE SER. THE SER SHALL HAVE THE OPPORTUNITY TO REVIEW THE PROPOSED SCHEDULE AND SUBMIT COMMENTS TO THE CONTRACTOR. THE FINAL SHOP DRAWING SCHEDULE SHALL BE DEVELOPED AND SUBMITTED TO THE SER. IN ACCORDANCE WITH THE SHOP DRAWING SCHEDULE, THE SER WILL RETURN THE SHOP DRAWINGS WITHIN TEN WORKING DAYS AFTER HAVING RECEIVED THE ELECTRONIC SHOP DRAWING.

SU-2 THE CONTRACTOR IS TO REVIEW EACH SUBMITTAL PRIOR TO FORWARDING TO DESIGN PROFESSIONALS. THE CONTRACTOR IS TO STAMP EACH SUBMITTAL VERIFYING THAT THE FOLLOWING IS ADDRESSED:

1. THE SHOP DRAWING IS REQUESTED.
2. THE SHOP DRAWING IS BASED ON THE LATEST DESIGN.
3. THE DESIGN PROFESSIONALS' COMMENTS FROM ANY PREVIOUS SUBMITTALS ARE ADDRESSED.
4. THE WORK IS COORDINATED AMONG ALL CONSTRUCTION TRADES.
5. REVISIONS FROM PREVIOUS SUBMITTALS ARE CLEARLY MARKED BY CIRCLING OR CLOUDS.
6. SUBMITTAL IS COMPLETE.
7. SUBMITTAL DOES NOT INCLUDE SUBSTITUTION REQUEST
8. SUBMITTAL SHALL INCLUDE A STAMP INDICATING PROJECT NAME AND LOCATION, SUBMITTAL NUMBER, SPECIFICATION SECTION NUMBER.

THE SER SHALL RETURN, WITHOUT COMMENT, SUBMITTALS WHICH THE CONTRACTOR HAS NOT STAMPED OR WHICH DO NOT MEET THE ABOVE REQUIREMENTS. THE SER'S REVIEW OF SUBMITTALS SHALL BE FOR GENERAL CONFORMANCE WITH THE DESIGN INTENT. NO WORK SHALL BE STARTED WITHOUT SUCH REVIEW.

SU-3 FOR COMPONENTS THAT REQUIRE ENGINEERING BY THE CONTRACTOR, PROVIDE A NOTE ON EACH SHOP DRAWING, WRITTEN AND SIGNED BY THE SUPPLIER'S ENGINEER, INDICATING THAT THE SHOP DRAWING IS IN CONFORMANCE WITH THE CALCULATIONS OF THE CONTRACTOR'S ENGINEER.

SU-4 THE FOLLOWING ITEMS REQUIRE SUBMITTALS FOR STRUCTURAL REVIEW AS OUTLINED IN THE SPECIFICATIONS:

039300	P		CONCRETE RESTORATION
051200	S		STRUCTURAL STEEL
053000	S		STEEL ROOF DECK
075323	P		EPDM ROOFING
078100	P		FIREPROOFING
112400	S	CALC	FALL PROTECTION EQUIPMENT
112400	S	CALC	TIE BACK ANCHOR LOAD TEST PLAN

S = SHOP DRAWINGS REQUIRED

P = PRODUCT DATA REQUIRED

CALC = SUPPORTING CALCULATIONS REQUIRED, SEALED AND SIGNED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE OF WISCONSIN.

SS STRUCTURAL STEEL

SS-1 STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS UNLESS OTHERWISE NOTED ON THE CONTRACT DOCUMENTS:

ROLLED SHAPES AND CHANNELS:	ASTM A572 OR A992, MIN YIELD STRENGTH 50 KSI
ANGLES:	ASTM A36
HOLLOW STRUCTURAL SECTIONS:	ASTM A500 GRADE C, MIN YIELD STRENGTH 46 KSI FOR ROUND AND 50 KSI FOR RECTANGULAR HSS
SEAMLESS PIPE:	ASTM A53 GRADE B, TYPE S, MIN YIELD STRENGTH 35 KSI.
PLATES	ASTM A572 OR A529, MIN YIELD STRENGTH 50 KSI

SS-2 CONNECTION MATERIAL SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS OR AS NEEDED FOR CONNECTION DESIGN:

ANGLES:	ASTM A36
WTs:	ASTM A992
PLATES:	ASTM A36, MIN YIELD STRENGTH 36 KSI OR ASTM A572 OR A529, MINIMUM YIELD STRENGTH 50 KSI
BOLTS:	ASTM A325 OR A490
NUTS:	ASTM A563
WASHERS:	ASTM F436
HEADED STUDS:	ASTM A 108, GRADE 1010 THROUGH 1020 HEADED STUD TYPE, COLD-FINISHED CARBON STEEL, AWS D1.1, TYPE B. 3/4" DIAMETER UNO
WELD ELECTRODES:	E70XX

SS-3 WHERE NO CAMBER IS INDICATED, FABRICATE BEAMS SO THAT ANY NATURAL CAMBER IS UPWARD AFTER ERECTION.

SS-4 SPLICES SHALL BE ALLOWED ONLY AT LOCATIONS SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS UNLESS APPROVED OTHERWISE BY THE SER IN WRITING.

SS-5 FOR STEEL MEMBERS AND EMBEDMENTS EXPOSED TO WEATHER, PROVIDE HOT-DIPPED GALVANIZED FINISH OR APPROVED ZINC RICH EXTERIOR COATING SYSTEM.

SS-6 PROVIDE HOLES IN ALL STEEL AS REQUIRED TO PREVENT ANY ACCUMULATION OF WATER. ALL PENETRATIONS THROUGH MAIN MEMBERS SHALL NOT EXCEED 1 1/8" DIA. AND SHALL BE GROUND SMOOTH. THESE DRAINS MUST BE KEPT CLEAN AND OPEN.

SS-7 SHOW ALL COPES, HOLES, OPENINGS AND MODIFICATIONS REQUIRED IN STRUCTURAL STEEL MEMBERS FOR ERECTION OR THE WORK OF OTHER TRADES ON THE SHOP DRAWINGS FOR APPROVAL BY THE DESIGN PROFESSIONALS.

SS-8 FIELD MODIFICATION OF STRUCTURAL STEEL IS PROHIBITED WITHOUT PRIOR WRITTEN APPROVAL OF THE DESIGN PROFESSIONALS.

SC STRUCTURAL STEEL CONNECTIONS

SC-1 ALL STEEL DETAILS AND CONNECTIONS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS", AISC-LOAD AND RESISTANCE FACTOR DESIGN.

SC-2 ALL CONNECTIONS INDICATED ON STRUCTURAL DRAWINGS HAVE BEEN COMPLETELY DESIGNED.

SC-3 DETAILS INDICATED ON DRAWINGS DO NOT SHOW ERECTION AIDS. PROVIDE ERECTION AIDS AS REQUIRED AND REMOVE THEM AFTER WORK IS COMPLETE.

SC-4 ALTERNATE CONNECTIONS TO THOSE SHOWN ON DRAWINGS WILL BE CONSIDERED AS A SUBSTITUTION REQUEST. SEE PROJECT SPECIFICATIONS.

SC-5 FOR CONNECTION DETAILING, SET CONNECTION WORK POINT AT INTERSECTION OF MEMBER CENTERLINES, UON.

SC-6 CONNECTION DESIGN FORCES INDICATED ON THE DRAWINGS ARE FACTORED UON.

SC-7 USE NO MORE THAN TWO BOLT DIAMETERS, ALL BOLTS OF THE SAME DIAMETER SHALL BE OF THE SAME GRADE, SKIP ONE SIZE BETWEEN DIAMETERS. BOLTS TO BE A MINIMUM OF 3/4-INCH DIAMETER GRADE A325.

SC-8 BEAM CONNECTION DESIGN NOTES:

EXCEPT WHERE "SNUG TIGHT" INSTALLATION IS SPECIFICALLY PERMITTED ON DRAWINGS OR "SLIP CRITICAL" DETAILING IS REQUIRED, ALL HIGH STRENGTH BOLTS SHALL BE INSTALLED AS FULL PRETENSIONED BOLTS.

AT A MINIMUM ALL BOLTED MOMENT AND AXIAL CONNECTION SHALL HAVE PRETENSIONED BOLTS IN STANDARD HOLES.

BOLTED MOMENT CONNECTIONS AT CANTILEVERS AND BACKSPANS SHALL USE SLIP CRITICAL BOLTS.

DO NOT USE OVERSIZED OR SLOTTED HOLES FOR ANY CONNECTIONS UNLESS SPECIFICALLY INDICATED ON THE DRAWINGS OR APPROVED IN WRITING BY THE SER.

SC-9 ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF THE STRUCTURAL WELDING CODE, ANSI/AWS D1.1, LATEST EDITION. ALL WELD SIZES SHALL BE THE LARGER OF THE SIZE REQUIRED BY CONNECTION FORCES, THE MINIMUM SIZE PER ANSI/AWS D1.1, OR 3/16 INCH MINIMUM FILLET WELD UON. ANY WELD SIZES SHOWN ON THE DESIGN DRAWINGS ARE CONSIDERED EFFECTIVE WELD SIZES AND SHALL BE INCREASED IN ACCORDANCE WITH AWS AS REQUIRED BY GAPS OR SKEWS BETWEEN COMPONENTS.

SC-10 USE RUNOFF TABS AT ALL BEVEL AND COMPLETE JOINT PENETRATION WELDS. REMOVE RUNOFF TABS BY NEAT CUTS AFTER WELD IS COMPLETED. GRIND SMOOTH WHERE REQUIRED BY DETAIL.

SC-11 WHERE REQUIRED BY DETAIL REMOVE WELD BACK UP BARS AND GRIND SMOOTH AFTER WELD IS COMPLETED.

SD STEEL DECK GENERAL REQUIREMENTS

SD-1 THE DESIGN, MANUFACTURE AND ERECTION OF STEEL DECK AND ITS ANCHORAGE SHALL, AT A MINIMUM, BE IN ACCORDANCE WITH "DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS AND ROOF DECKS" OF THE STEEL DECK INSTITUTE (SDI), CURRENT EDITION AND "SPECIFICATIONS FOR DESIGN OF LIGHT GAGE COLD FORMED STEEL STRUCTURAL MEMBERS" AS PUBLISHED BY THE AMERICAN IRON AND STEEL INSTITUTE (AISI), CURRENT EDITION.

SD-2 CONFIGURE ALL STEEL DECK USING THREE SPAN CONTINUOUS LAYOUTS WHEREVER POSSIBLE.

SD-3 CONFIGURE ALL STEEL DECK AS SHOWN ON THE DRAWINGS.

SD-4 DESIGN STEEL DECK FOR UNSHORED CONDITIONS.

RD STEEL ROOF DECK

RD-1 STEEL ROOF DECK SHALL CONFORM TO THE FOLLOWING STANDARDS AND MATERIAL PROPERTIES:

STEEL FOR DECK ASTM A653, MINIMUM YIELD STRENGTH OF 33 KSI
HOT-DIP GALVANIZING ASTM A653 G60

ROOF DECK SHALL BE HOT-DIP GALVANIZED, UON

RD-2 ROOF DECK ANCHORAGE TO SUPPORTING MEMBERS SHALL MEET THE FOLLOWING MINIMUM FASTENING REQUIREMENTS:
A. AT ENDS OF UNITS AND AT ALL INTERMEDIATE SUPPORTS: BY PUDDLE WELDS NOT LESS THAN 5/8 INCH DIAMETER SPACED NOT MORE THAN 12 INCHES ON CENTER MAX.
B. SIDE LAPS OF ADJACENT UNITS: SHALL BE FASTENED BY #10 TEK SCREWS SPACED AT 6" OC.

RD-3 DECKING CONTRACTOR SHALL COORDINATE DECK OPENING SIZES AND LOCATIONS FROM ARCHITECTURAL AND MEP CONTRACT DOCUMENTS, PROVIDE HEADER MEMBERS OR REINFORCEMENT AS REQUIRED BY TYPICAL DETAILS EVEN IF NOT SHOWN ON THE PLANS, AND SUBMIT PROPOSED OPENINGS THROUGH SLAB/DECK FOR REVIEW BY THE DESIGN PROFESSIONALS.

PA POST-INSTALLED ANCHORS

PA-1 FIELD DRILLED EXPANSION ANCHOR SYSTEMS USED FOR DESIGN:

HILTI KWIK BOLT TZ

PA-2 PROOF TESTING OF EXPANSION ANCHORS SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS. UNLESS NOTED OTHERWISE, EXPANSION ANCHOR PROOF TORQUE LOADS SHALL BE PER THE EXPANSION ANCHOR PROOF SCHEDULES.

PA-3 ALTERNATIVE SYSTEM EQUIVALENT TO OR EXCEEDING THE PROPERTIES OF THE SYSTEMS ABOVE WILL BE CONSIDERED AS A SUBSTITUTION REQUEST. SEE PROJECT SPECIFICATIONS.

PA-4 INSTALL ANCHORS TO MEET THE REQUIREMENTS INDICATED IN THE CONTRACT DOCUMENTS AND THE CURRENT MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS (MPII).

PA-5 LOCATE, BY NON-DESTRUCTIVE MEANS, AND AVOID ALL EXISTING REINFORCEMENT PRIOR TO INSTALLATION OF ANCHORS. IF EXISTING REINFORCING LAYOUT PROHIBITS THE INSTALLATION OF ANCHORS AS INDICATED ON THE DRAWINGS, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE DESIGN PROFESSIONALS.

PA-6 SEE PROJECT SPECIFICATIONS FOR POST-INSTALLED ANCHOR INSPECTION REQUIREMENTS.

FP FIREPROOFING

FP-1 WHERE EXISTING FIREPROOFING IS REMOVED TO INSTALL NEW FRAMING OR DECK, RE-APPLY FIREPROOFING TO EXPOSED PORTIONS OF EXISTING FRAMING AS WELL AS TO NEW FRAMING AND TO THE UNDERSIDE OF NEW DECK. THICKNESS FIREPROOFING AS REQUIRED TO ACHIEVE 2-HOUR RATING.

TB TIE BACK ANCHORS

TB-1 EACH TIE BACK ANCHOR HAS BEEN DESIGNED FOR A SINGLE FALL ARREST LOAD OF 5,000 LB CAPABLE OF ACTING IN ANY DIRECTION.

TB-2 IN AN ANCHOR GROUP, ONE TIE BACK ANCHOR IS TO BE USED FOR THE ROPE DESCENT SYSTEM AND ONE IS TO BE USED FOR THE FALL ARREST SYSTEM. THE BASE BUILDING STRUCTURE HAS BEEN EVALUATED FOR ONE FALL ARREST LOAD PER ANCHOR GROUP. THE BASE BUILDING STRUCTURE HAS NOT BEEN EVALUATED FOR A FALL ARREST LOAD OCCURING SIMULTANEOUSLY AT BOTH ANCHORS IN A GROUP. SEE FIGURE 1 BELOW.

TB-3 ALL NEW TIE BACK ANCHOR ASSEMBLIES SHALL BE TESTED IN THEIR FINAL AS-BUILT CONDITION PRIOR TO BEING PLACED INTO SERVICE IN ACCORDANCE WITH OSHA 1910.66(g)(1) FOR A FALL ARREST PROOF LOAD OF 5,000 LBS. REFER TO SPECIFICATION FOR ADDITIONAL TESTING PROCEDURE REQUIREMENTS.

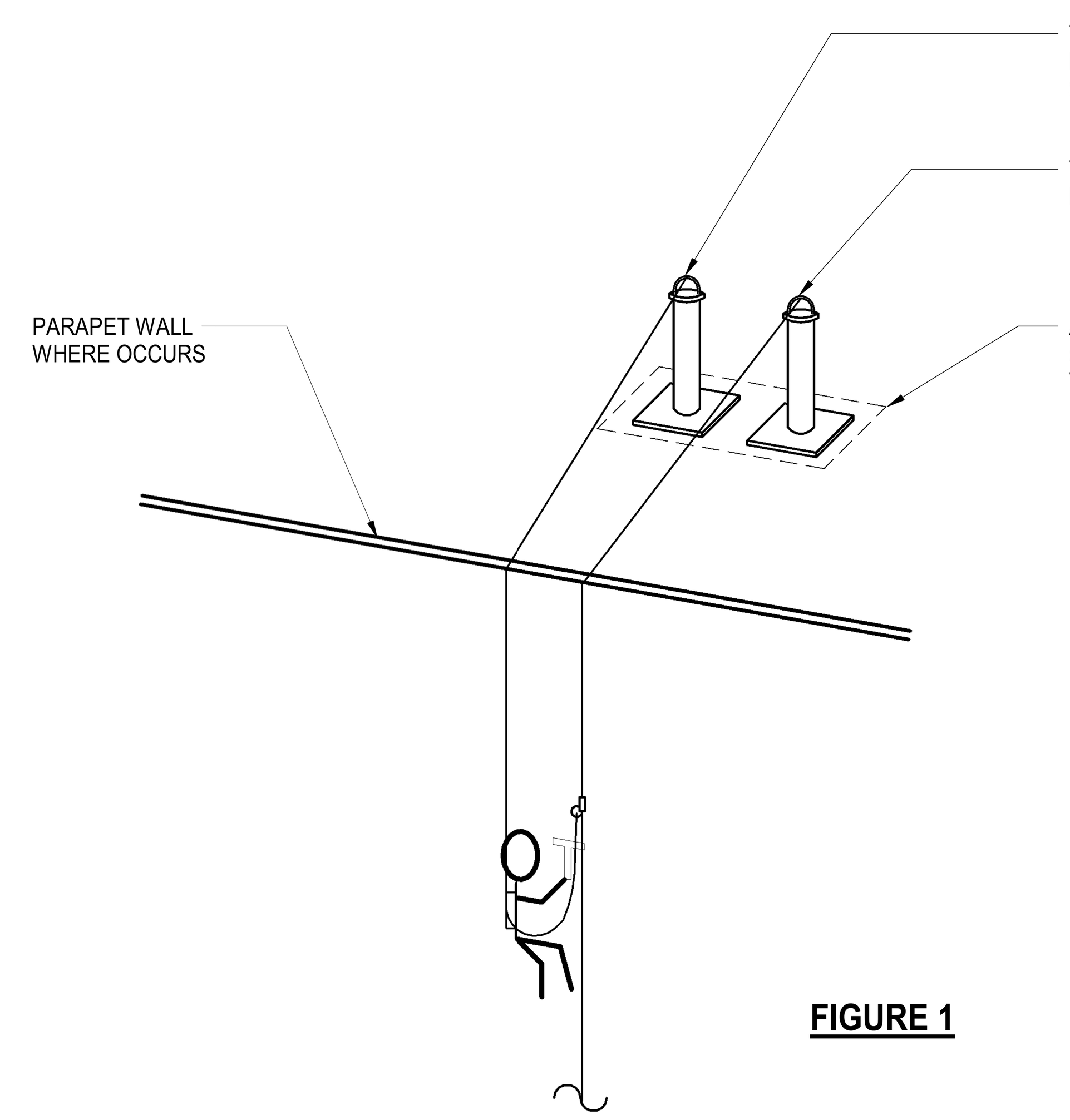

FIGURE 1

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12/30/2019
Exp. 7.31.2020

REV	ISSUE	DATE
1	80% CDs	11/05/19
2	CONSTRUCTION DOCUMENTS	12/30/19

NEW REMOTE DESCENT SYSTEM ANCHORAGE

CITY-COUNTY BUILDING
210 MARTIN LUTHER KING JR BLVD
MADISON, WI 53703

TITLE
GENERAL NOTES

PROJECT NO: C19150.00
SCALE: 3/4" = 1'-0"
DRAWN BY: JRC
CHECKED BY: RTM
DATE: 11/05/2019

SHEET NUMBER
S-001

ABBREVIATION	DESCRIPTION
ADDL	ADDITIONAL
ADJ	ADJACENT
ALT	ALTERNATE
APPRX	APPROXIMATE
ARCH	ARCHITECT OR ARCHITECTURAL
B/	BOTTOM OF
B/B	BACK TO BACK
BAL	BALANCE
BLDG	BUILDING
BLK	BLOCK
BLKG	BLOCKING
BM	BEAM
BOT	BOTTOM
BRDG	BRIDGING
BRG	BEARING
BTWN	BETWEEN
C	COMPRESSION
C/C	CENTER TO CENTER
CIP	CAST-IN-PLACE
CL	CENTER LINE
CLR	CLEAR OR CLEARANCE
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
COMP	COMPRESSION
CONC	CONCRETE
CONN	CONNECTION(S)
CONST	CONSTRUCTION
CONT	CONTINUOUS
db	REINFORCING BAR DIAMETER
DBL	DOUBLE
DCW	DEMAND CRITICAL WELD
DEG	DEGREE(S)
DET	DETAIL
DIA	DIAMETER
DIAG	DIAGONAL
DIM(S)	DIMENSION(S)
DL	DEAD LOAD
DWG(S)	DRAWING(S)
DWL	DOWEL(S)
EA	EACH
ECC	ECCENTRICITY
EF	EACH FACE
EL	ELEVATION
ELEC	ELECTRICAL
ENGR	ENGINEER
EOD	EDGE OF DECK
EOS	EDGE OF SLAB
EQ	EQUAL
EQUIP	EQUIPMENT
EW	EACH WAY
EXP	EXPANSION
EXST	EXISTING
EXT	EXTERIOR
F/F	FACE TO FACE
FIN	FINISH(ED)
FLR	FLOOR
FND	FOUNDATION
FP	FIREPROOF(ING)
FS	FAR SIDE
FTG	FOOTING
GA	GAGE, GAUGE
GALV	GALVANIZED
GB	GRADE BEAM
GEN	GENERAL
GR	GRADE
HK	HOOK
HORIZ	HORIZONTAL
HP	HIGH POINT
HT	HEIGHT
ID	INSIDE DIAMETER
IF	INSIDE FACE
INFO	INFORMATION
INT	INTERIOR
INTRM	INTERMEDIATE
JST(S)	JOIST(S)
JT	JOINT
K	KIPS (1,000 POUNDS)
KLF	KIP PER LINEAR FOOT
KSF	KIP PER SQUARE FOOT
LL	LIVE LOAD

ABBREVIATION	DESCRIPTION
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
LP	LOW POINT
LW	LIGHTWEIGHT
LWC	LIGHTWEIGHT CONCRETE
M	MOMENT
MATL	MATERIAL
MAX	MAXIMUM
MC	MOMENT CONNECTION(S)
MECH	MECHANICAL
MEP	MECHANICAL, ELECTRICAL, PLUMBING, FIRE PROTECTION
MEZZ	MEZZANINE
MFR	MANUFACTURER
MID	MIDDLE
MIN	MINIMUM
MISC	MISCELLANEOUS
NIC	NOT IN CONTRACT
NO	NUMBER
NOM	NOMINAL
NS	NEAR SIDE
NTS	NOT TO SCALE
NW	NORMAL WEIGHT
NWC	NORMALWEIGHT CONCRETE
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OH	OPPOSITE HAND
OPNG(S)	OPENING(S)
OPP	OPPOSITE
OSL	OUTSTANDING LEG
PC	PIECE
PCY	POUNDS PER CUBIC YARD
PERP	PERPENDICULAR
PG	PLATE GIRDER
PL	PLATE
PRC	PRECAST
PRLL	PARALLEL
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	POINT OR POST-TENSION(ED) OR (ING)
RAD	RADIUS
REF	REFERENCE
REINF	REINFORCE(D) (ING) OR (MENT)
REQD	REQUIRED
S&T	SHRINKAGE AND TEMPERATURE
SCHED	SCHEDULE(D)
SDL	SUPERIMPOSED DEAD LOAD
SECT	SECTION
SER	STRUCTURAL ENGINEER OF RECORD
SF	SQUARE FOOT (FEET)
SHT	SHEET
SIM	SIMILAR
SLRS	SEISMIC LOAD RESISTING SYSTEM
SOG	SLAB ON GRADE
SP	SPACE
SPEC(S)	SPECIFICATION(S)
STD	STANDARD
STL	STEEL
STR	STRUCTURE
STRCTL	STRUCTURAL
SYM	SYMMETRICAL
T	TENSION
T&B	TOP AND BOTTOM
T/	TOP OF
TEMP	TEMPERATURE OR TEMPORARY
TEN	TENSION
THK	THICK OR THICKNESS
TYP	TYPICAL
UON	UNLESS OTHERWISE NOTED
V	SHEAR
VERT	VERTICAL
VIF	VERIFY IN FIELD
W/	WITH
W/O	WITHOUT
WD	WOOD
WP	WORK POINT
WPFG	WATERPROOFING
WS	WATERSTOP
WWR	WELDED WIRE REINFORCEMENT

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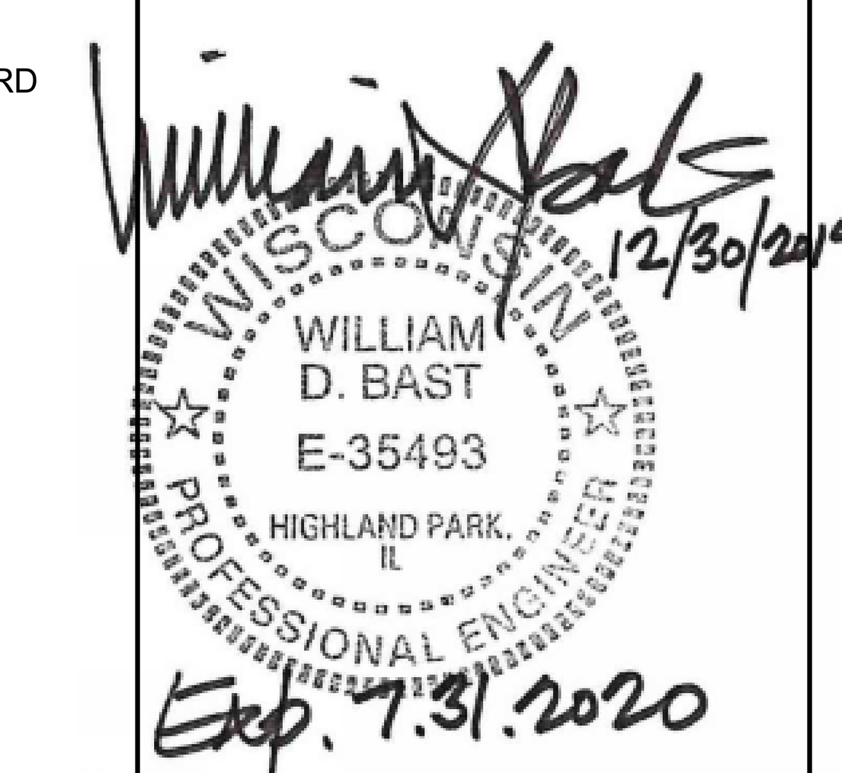

REV	ISSUE	DATE
1	80% CDs	11/05/19
2	CONSTRUCTION DOCUMENTS	12/30/19

NEW REMOTE DESCENT SYSTEM ANCHORAGE

CITY-COUNTY BUILDING
210 MARTIN LUTHER KING JR BLVD
MADISON, WI 53703

TITLE
ABBREVIATIONS

PROJECT NO: C19150.00
SCALE
DRAWN BY: JRC
CHECKED BY: RTM
DATE: 11/05/2019

SHEET NUMBER
S-002

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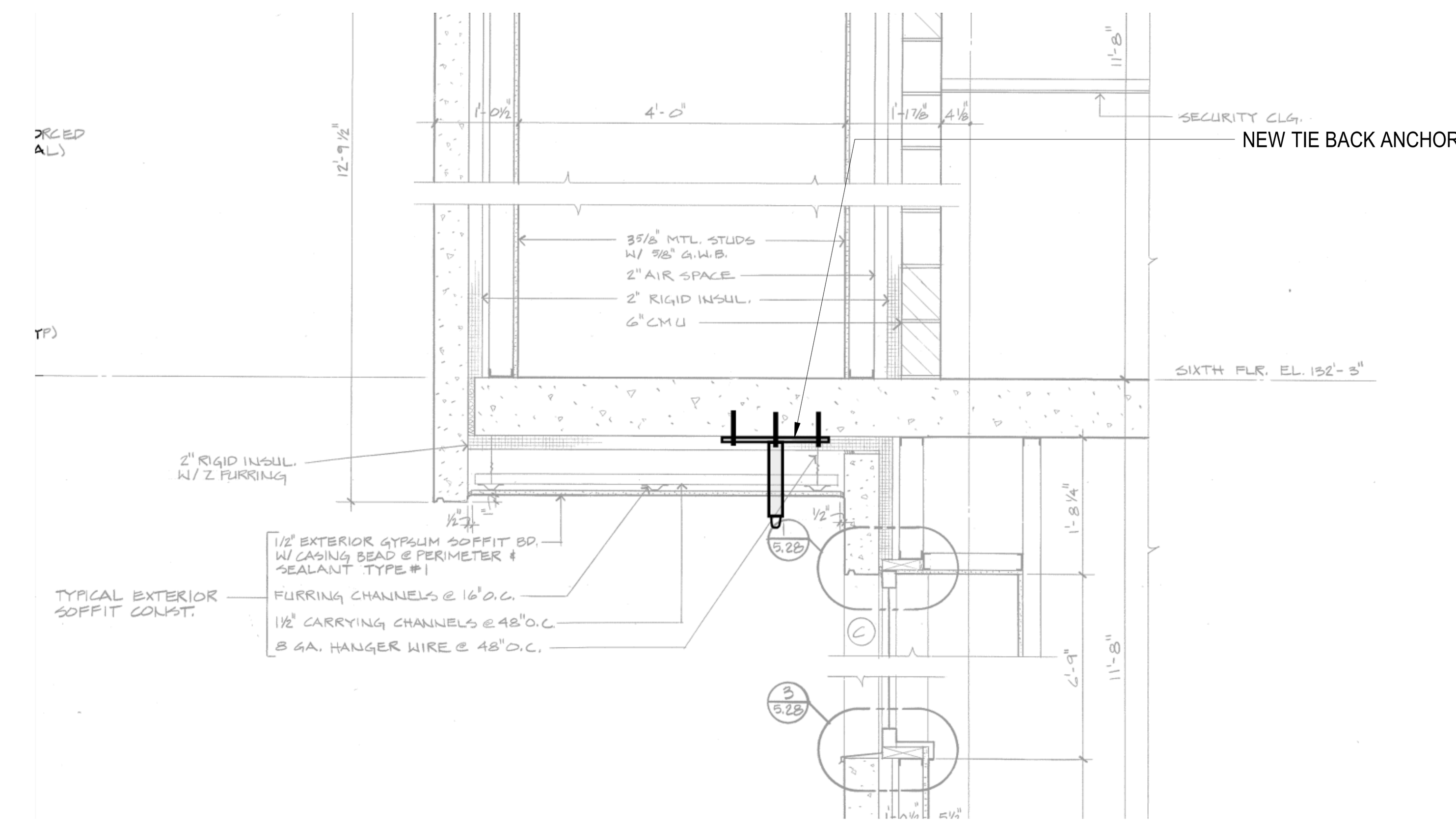
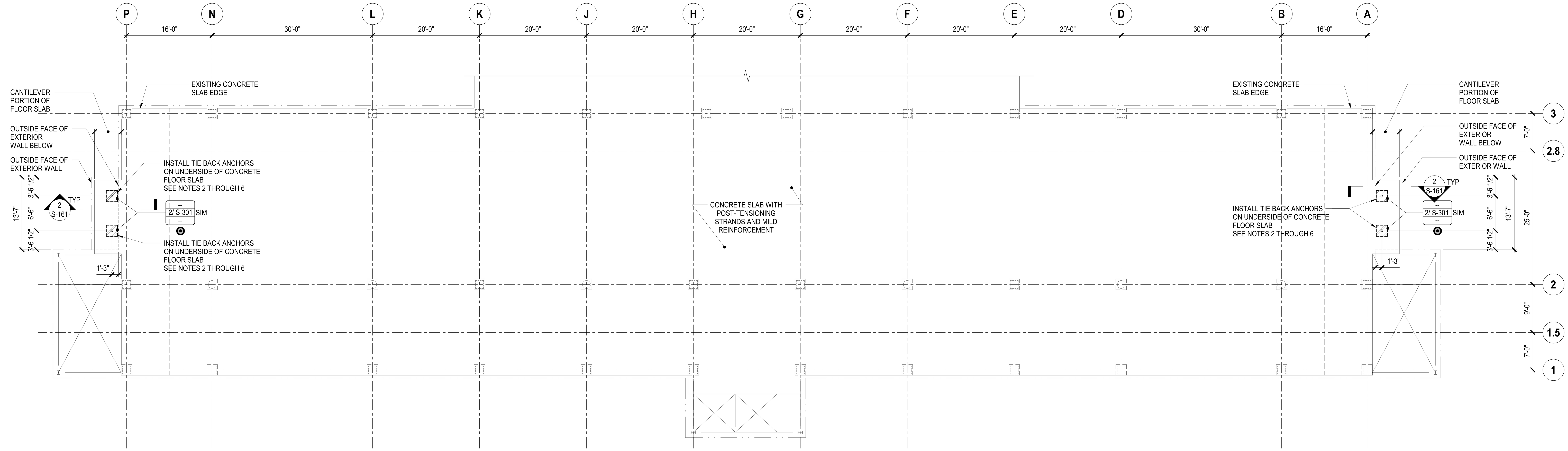


2 SECTION AT SIXTH FLOOR OVERHANG

NOT TO SCALE



1 SIXTH FLOOR PARTIAL PLAN - WEST (ADD ALTERNATE)

SCALE: 1/8" = 1'-0"

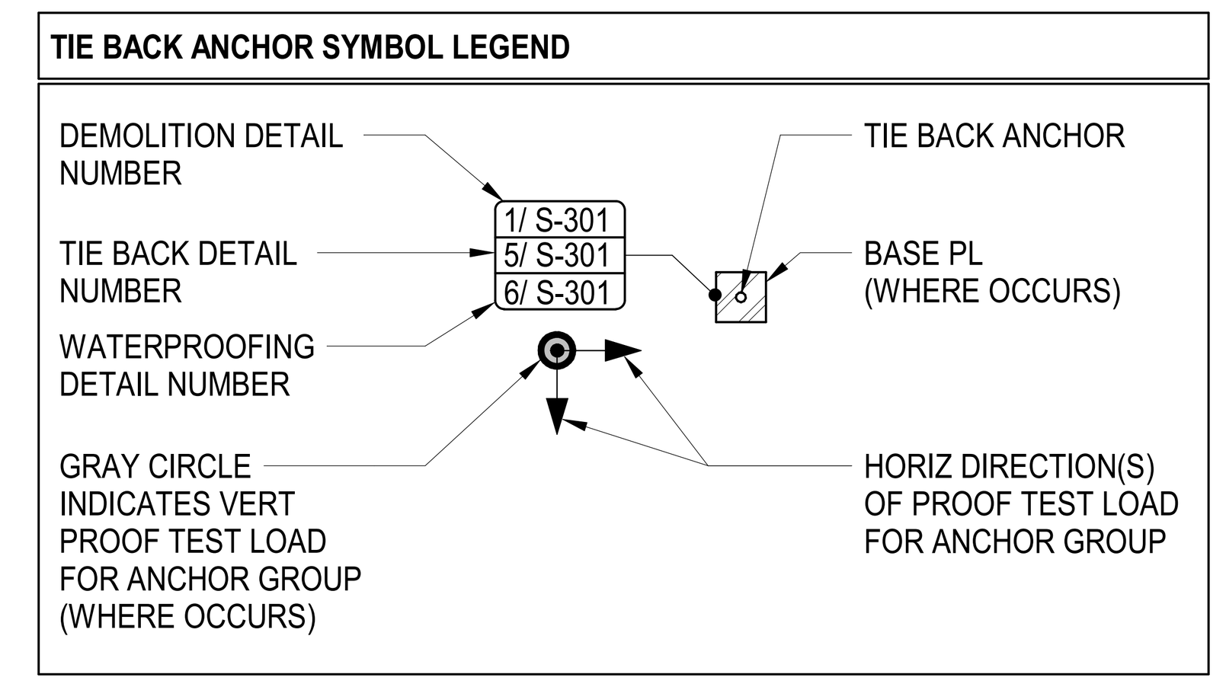
PLAN NOTES:

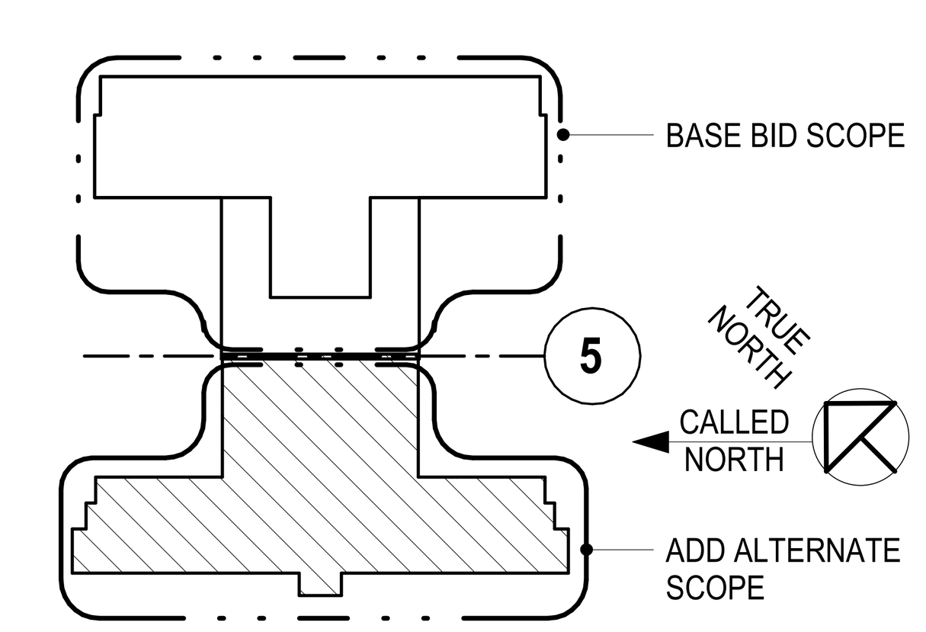
1. TOP OF SLAB ELEVATION = 132'-3" (VERIFY IN FIELD). TOP OF STEEL ELEVATION VARIES.
2. REMOVE EXISTING SOFFIT SHEATHING AND RIGID INSULATION AS REQUIRED TO INSTALL NEW TIE BACK ANCHORS.
3. INSTALL NEW 2" XPS INSULATION TO CONCEAL ANCHOR BASE PLATE. REPLACE REMOVED SOFFIT SHEATING WITH USG SECUROCK BRAND GLASS-MAT SHEATHING (OR SIMILAR). THICKNESS TO MATCH EXISTING. INSTALL ADDITIONAL FURRING CHANNELS, CARRYING CHANNELS AND HANGERS AS REQUIRED TO ACCOMMODATE NEW TIE BACK ANCHORS. OPENING IN SOFFT SHEATHING FOR ANCHORS TO BE 1" LARGER THAN THE DIAMETER OF THE ANCHOR POST. INSTALL URETHANE SEALANT OVER BACKER ROD IN GAP. SEALANT COLOR TO MATCH EXISTING.
4. FASTEN SOFFIT SHEATHING TO FURRING CHANNELS WITH 1 1/4" #6 BUGLEHEAD CORROSION-RESISTANT SCREWS AT 8" OC MAX.
5. PROVIDE 1/2" GAP BETWEEN SOFFIT SHEATHING AND PRECAST WALL PANELS. INSTALL URETHANE SEALANT OVER BACKER ROD IN GAPS. SEALANT COLOR TO MATCH EXISTING.
6. SOFFIT SHEATHING FINISH:

   APPLY FILL COAT OF USG SHEETROCK BRAND DURABOND SETTING-TYPE JOINT COMPOUND (OR SIMILAR) AND EMBED USG SHEETROCK BRAND JOINT TAPE (OR SIMILAR) OVER ALL JOINTS. ONCE FILL COAT IS DRY, APPLY FINISH COAT OF JOINT COMPOUND OVER JOINTS.

   APPLY A SMOOTH, UNIFORM THIN COAT OF USG SHEETROCK BRAND DURABOND SETTING-TYPE JOINT COMPOUND (OR SIMILAR) OVER THE ENTIRE SURFACE OF THE SHEATHING. AFTER COMPOUND HAS DRIED, APPLY LATEX FLAT EXTERIOR PRIMER AND FINISH WITH TWO COATS OF LATEX EXTERIOR PAINT. COLOR TO MATCH EXISTING.
7. LOCATE EXISTING CONCRETE SLAB POST-TENSIONING STRANDS AND MILD REINFORCEMENT VIA NON-DESTRUCTIVE TESTING PRIOR TO DRILLING HOLES PER GENERAL STRUCTURAL NOTE PA-5.

TIE BACK ANCHOR SYMBOL LEGEND





KEYPLAN

WISCONSIN WILLIAM D. BAST E-35493 HIGHLAND PARK, IL PROFESSIONAL ENGINEER
12/30/2019
Exp. 7.31.2020

REV	ISSUE	DATE
2	CONSTRUCTION DOCUMENTS	12/30/19

NEW REMOTE DESCENT SYSTEM ANCHORAGE

CITY-COUNTY BUILDING
210 MARTIN LUTHER KING JR BLVD
MADISON, WI 53703

TITLE
SIXTH FLOOR PARTIAL PLAN - WEST

PROJECT NO. C19150.00
SCALE As indicated
DRAWN BY Author
CHECKED BY Checker
DATE 12/19/19

SHEET NUMBER **S-161**

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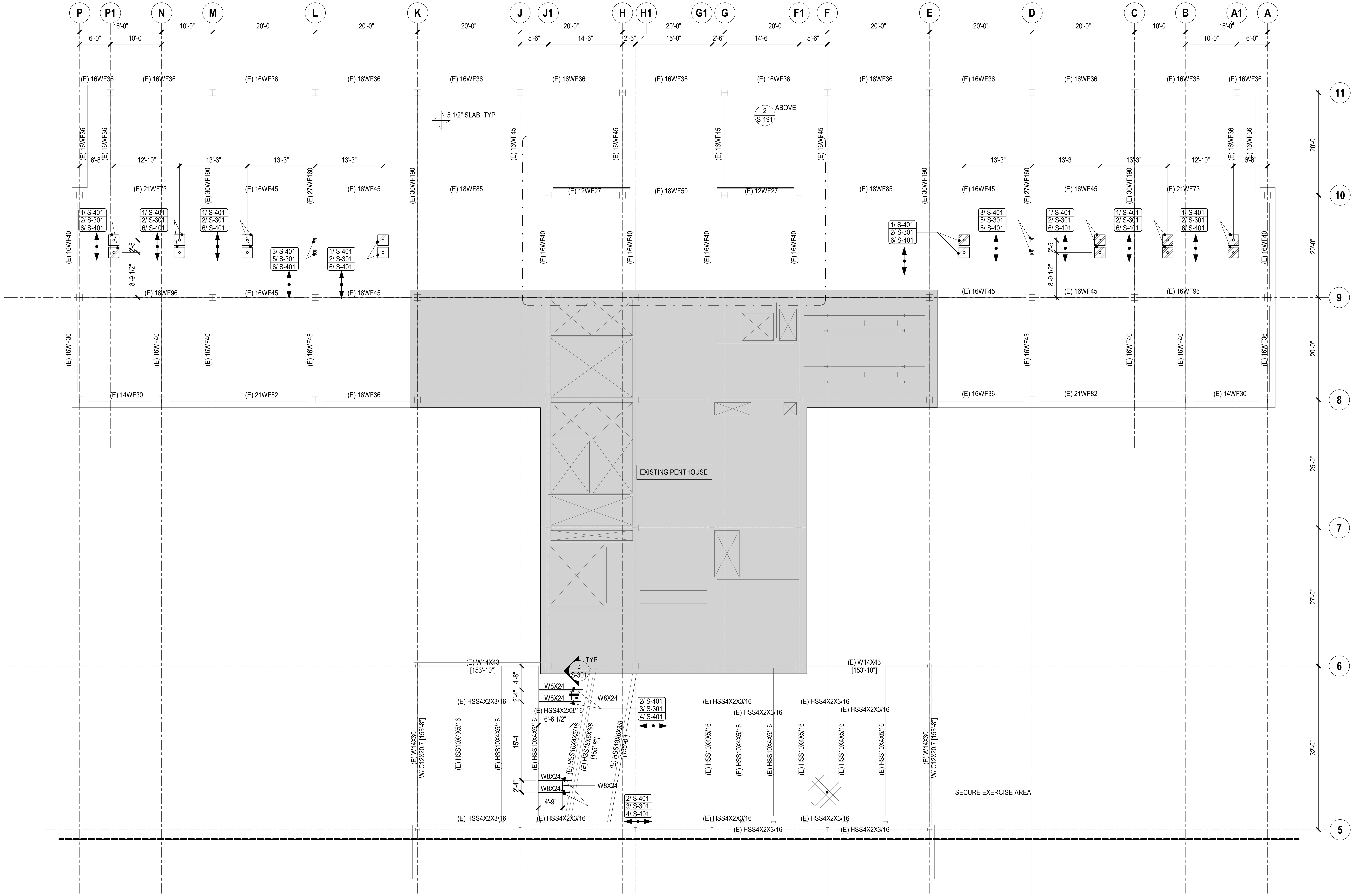


1 MAIN ROOF PLAN - EAST (BASE BID)

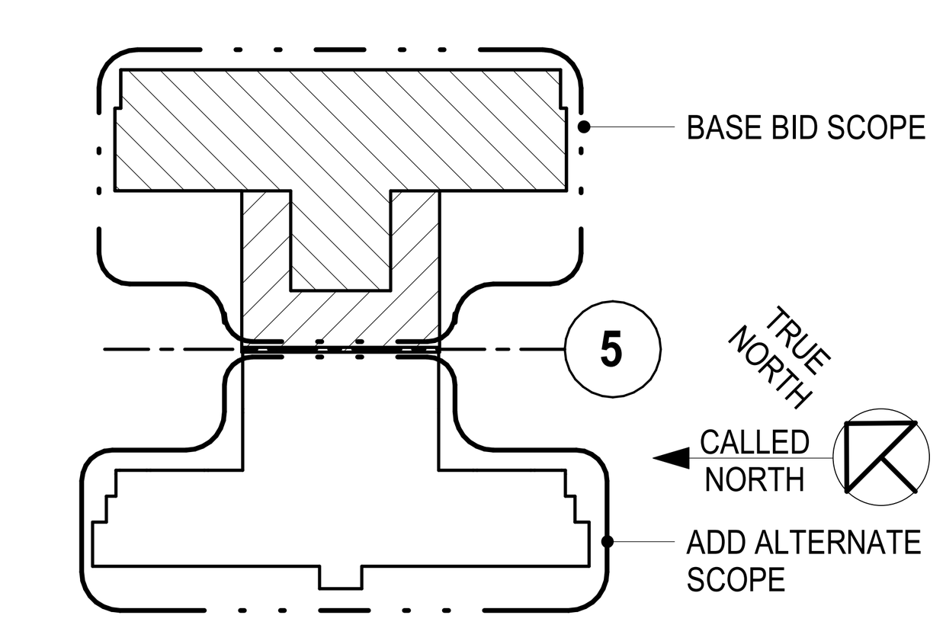
SCALE: 1/8" = 1'-0"

PLAN NOTES:

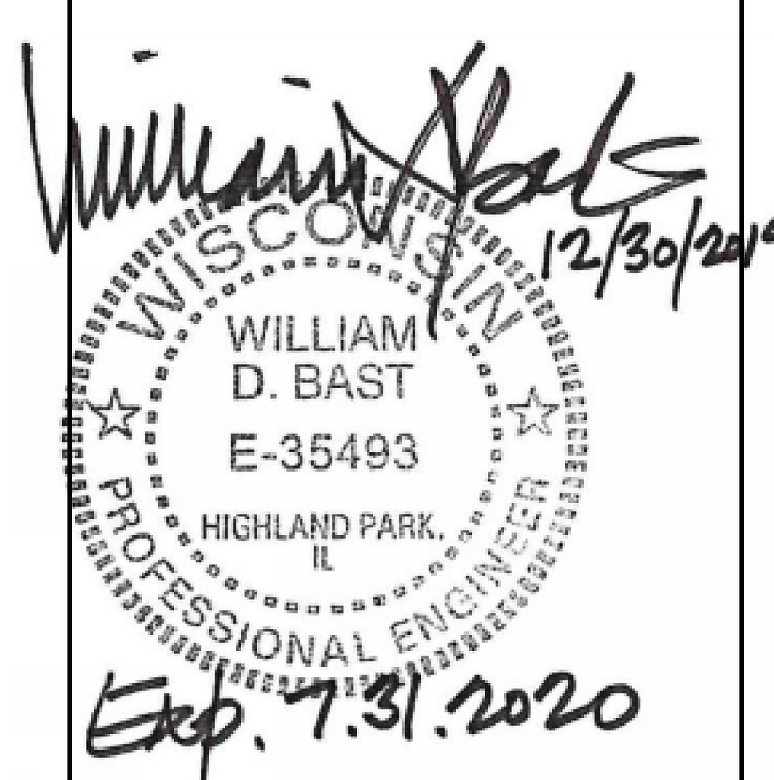
1. TOP OF STRUCTURAL SLAB EL = +155'-4", UON.
   TOP OF STRUCTURAL STEEL EL = +155'-2", UON.
2. LOCATE EXISTING CONCRETE SLAB REINFORCEMENT VIA NON-DESTRUCTIVE TESTING PRIOR TO DRILLING HOLES PER GENERAL STRUCTURAL NOTE PA-5.

TIE BACK ANCHOR SYMBOL LEGEND

DEMOLITION DETAIL NUMBER
TIE BACK DETAIL NUMBER
WATERPROOFING DETAIL NUMBER
GRAY CIRCLE INDICATES VERT PROOF TEST LOAD FOR ANCHOR GROUP (WHERE OCCURS)
1/ S-301
5/ S-301
6/ S-301
TIE BACK ANCHOR
BASE PL (WHERE OCCURS)
HORIZ DIRECTION(S) OF PROOF TEST LOAD FOR ANCHOR GROUP



KEYPLAN



REV	ISSUE	DATE
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2	CONSTRUCTION DOCUMENTS	12/30/19

NEW REMOTE DESCENT SYSTEM ANCHORAGE

CITY-COUNTY BUILDING
210 MARTIN LUTHER KING JR BLVD
MADISON, WI 53703

TITLE
MAIN ROOF PLAN - EAST

PROJECT NO: C19150.00
SCALE: As indicated
DRAWN BY: JRC
CHECKED BY: RTM
DATE: 11/05/2019

SHEET NUMBER
S-181

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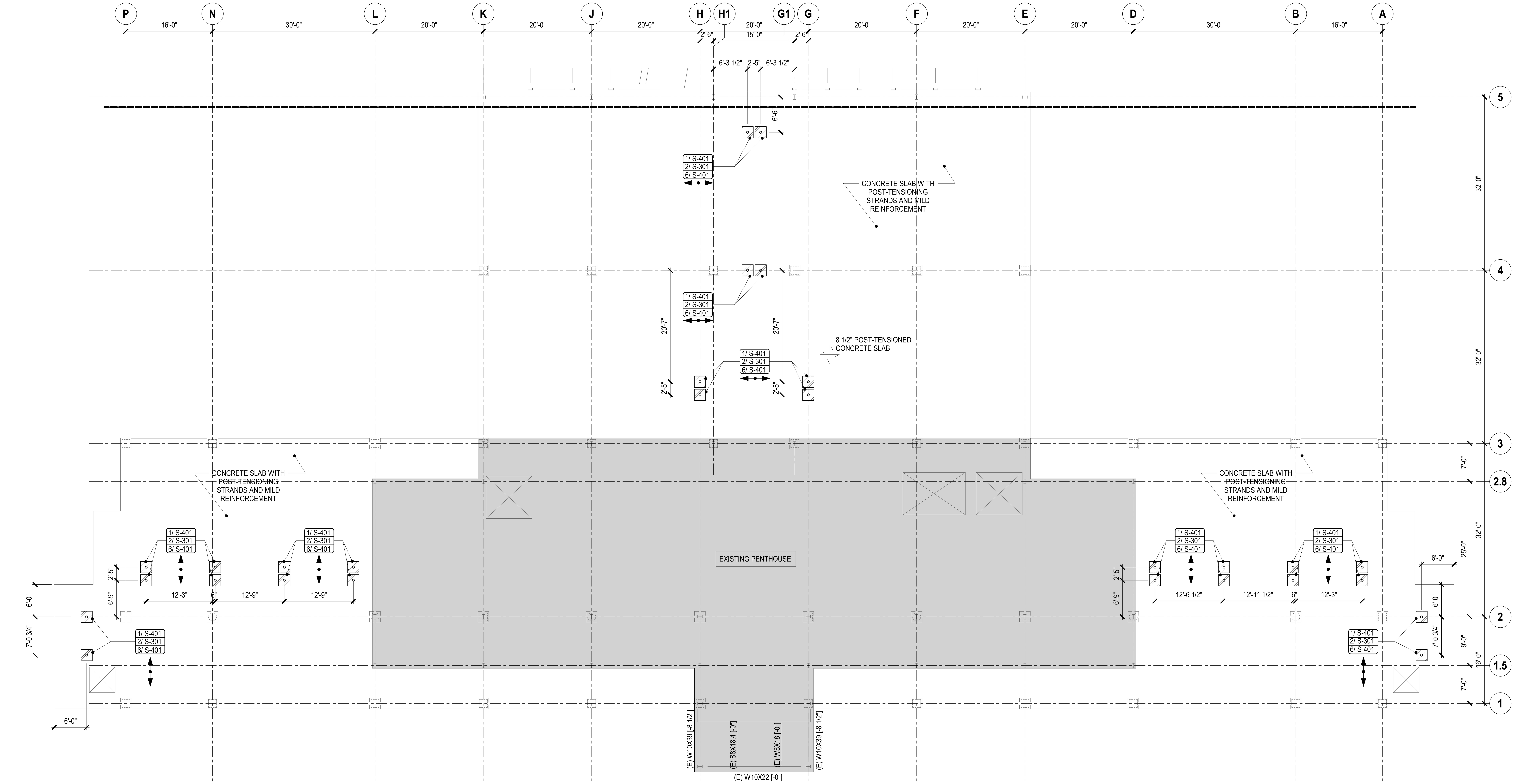


1 MAIN ROOF PLAN - WEST (ADD ALTERNATE)

SCALE: 1/8" = 1'-0"

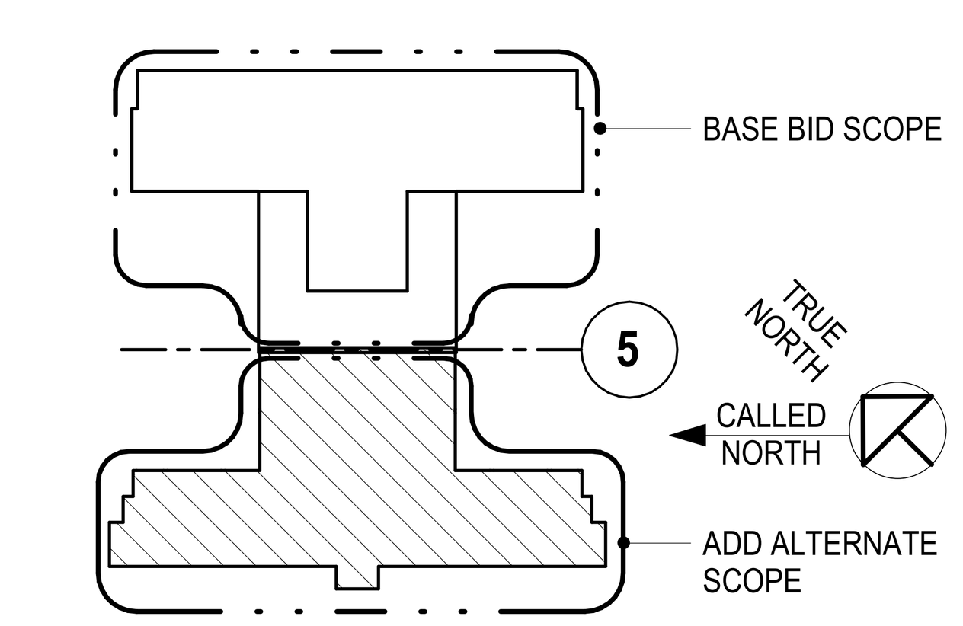
PLAN NOTES:

1. TOP OF STRUCTURAL SLAB AT EL = +156'-8", UON.
2. LOCATE EXISTING CONCRETE SLAB POST-TENSIONING STRANDS AND MILD REINFORCEMENT VIA NON-DESTRUCTIVE TESTING PRIOR TO DRILLING HOLES PER GENERAL STRUCTURAL NOTE PA-5.

TIE BACK ANCHOR SYMBOL LEGEND

- DEMOLITION DETAIL NUMBER
- TIE BACK DETAIL NUMBER
- WATERPROOFING DETAIL NUMBER
- GRAY CIRCLE INDICATES VERT PROOF TEST LOAD FOR ANCHOR GROUP (WHERE OCCURS)
- TIE BACK ANCHOR
- BASE PL (WHERE OCCURS)
- HORIZ DIRECTION(S) OF PROOF TEST LOAD FOR ANCHOR GROUP

1/ S-301
5/ S-301
6/ S-301



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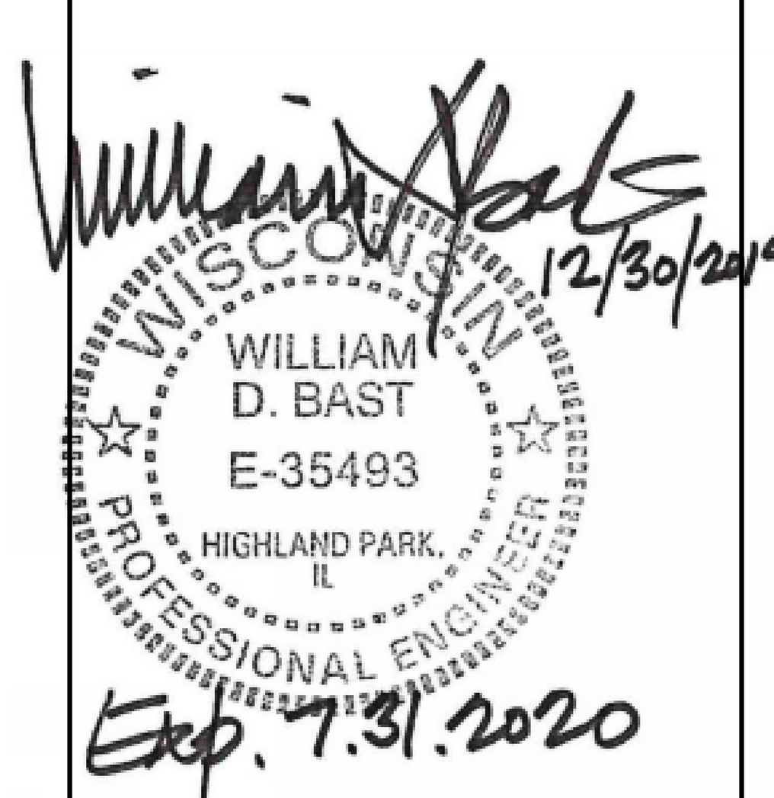

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2	CONSTRUCTION DOCUMENTS	12/30/19

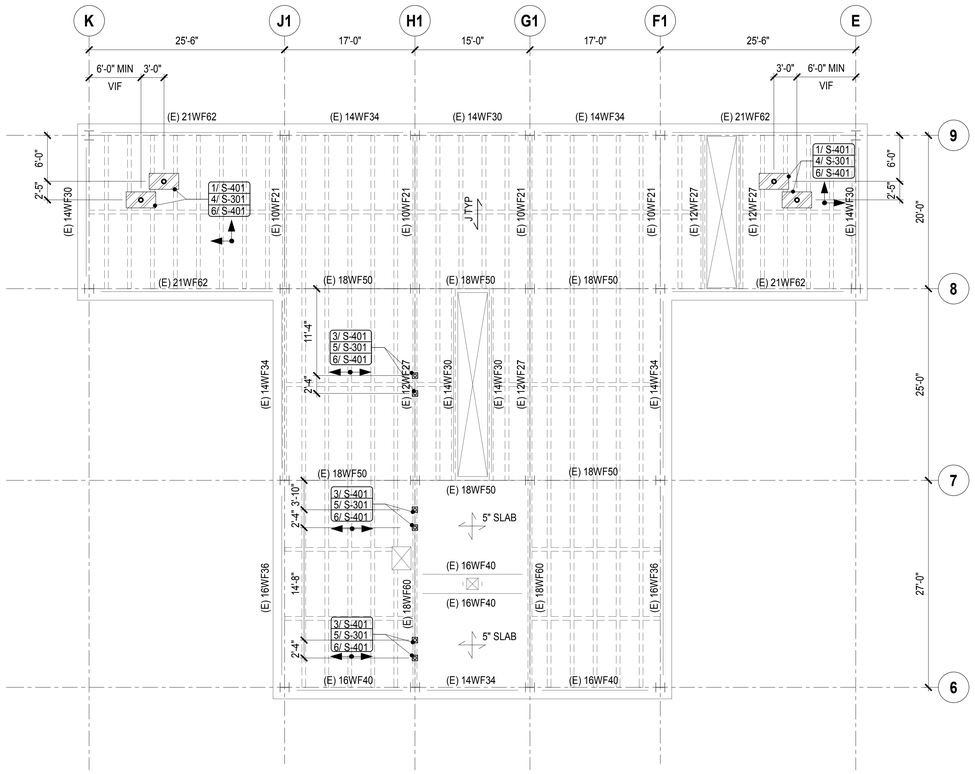
NEW REMOTE DESCENT SYSTEM ANCHORAGE

CITY-COUNTY BUILDING
210 MARTIN LUTHER KING JR BLVD
MADISON, WI 53703

TITLE
MAIN ROOF PLAN - WEST

PROJECT NO: C19150.00
SCALE: As indicated
DRAWN BY: JRC
CHECKED BY: RTM
DATE: 11/05/2019

SHEET NUMBER
S-182



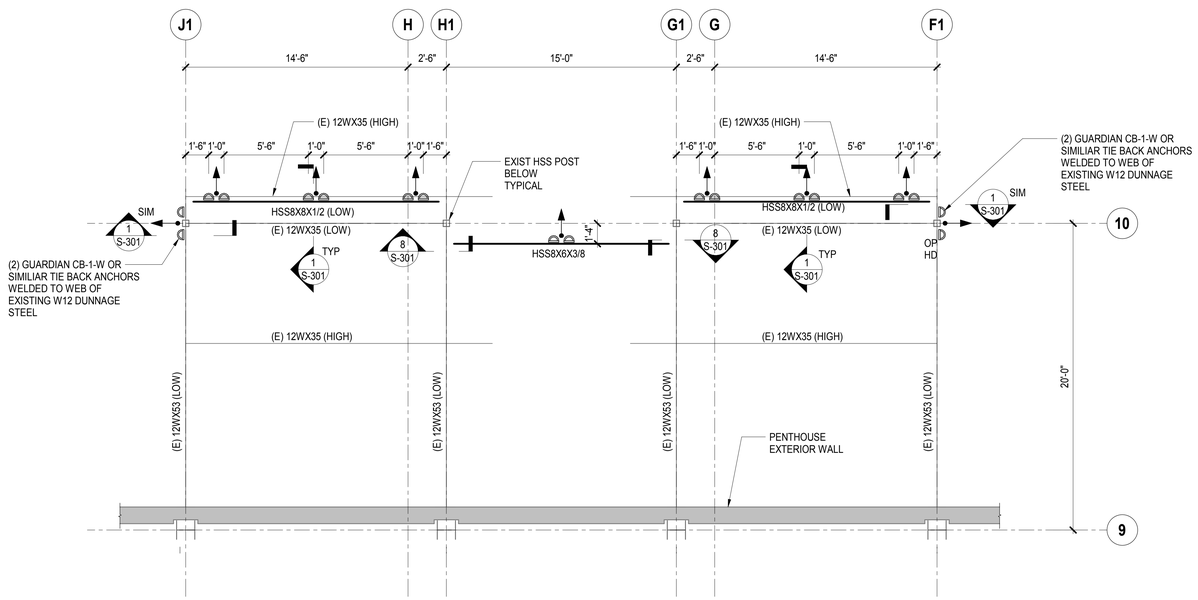
PLAN NOTES:

1. TOP OF STRUCTURAL SLAB EL = +172'-10", UON.
   TOP OF STRUCTURAL STEEL EL = +172'-8", UON.

2. LOCATE EXISTING CONCRETE SLAB POST-TENSIONING STRANDS AND MILD REINFORCEMENT VIA NON-DESTRUCTIVE TESTING PRIOR TO DRILLING HOLES PER GENERAL STRUCTURAL NOTE PA-5.

1 PENTHOUSE ROOF PLAN - EAST (BASE BID)

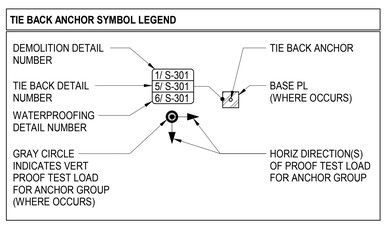
SCALE: 1/8" = 1'-0"

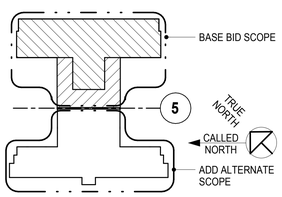


2 EAST COOLING TOWER DUNNAGE STEEL FRAMING PLAN (BASE BID)

SCALE: 1/4" = 1'-0"

TIE BACK ANCHOR SYMBOL LEGEND





KEYPLAN

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12/30/2019

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2	CONSTRUCTION DOCUMENTS	12/30/19

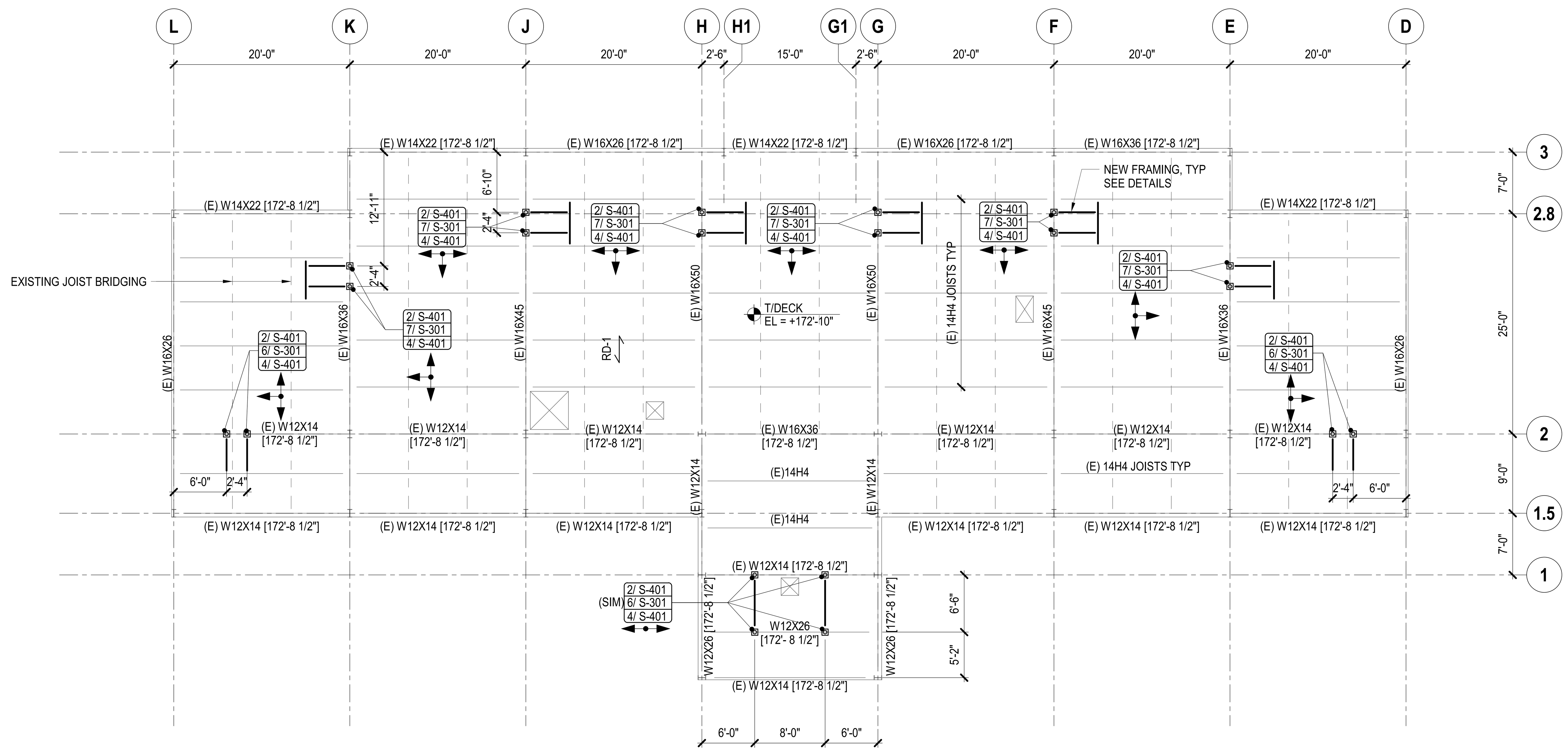
NEW REMOTE DESCENT SYSTEM ANCHORAGE

CITY-COUNTY BUILDING
210 MARTIN LUTHER KING JR BLVD
MADISON, WI 53703

TITLE
PENTHOUSE ROOF PLAN - EAST

PROJECT NO: **C19150.00**
SCALE: **As indicated**
DRAWN BY: **JRC**
CHECKED BY: **RTM**
DATE: **11/05/2019**

SHEET NUMBER
S-191



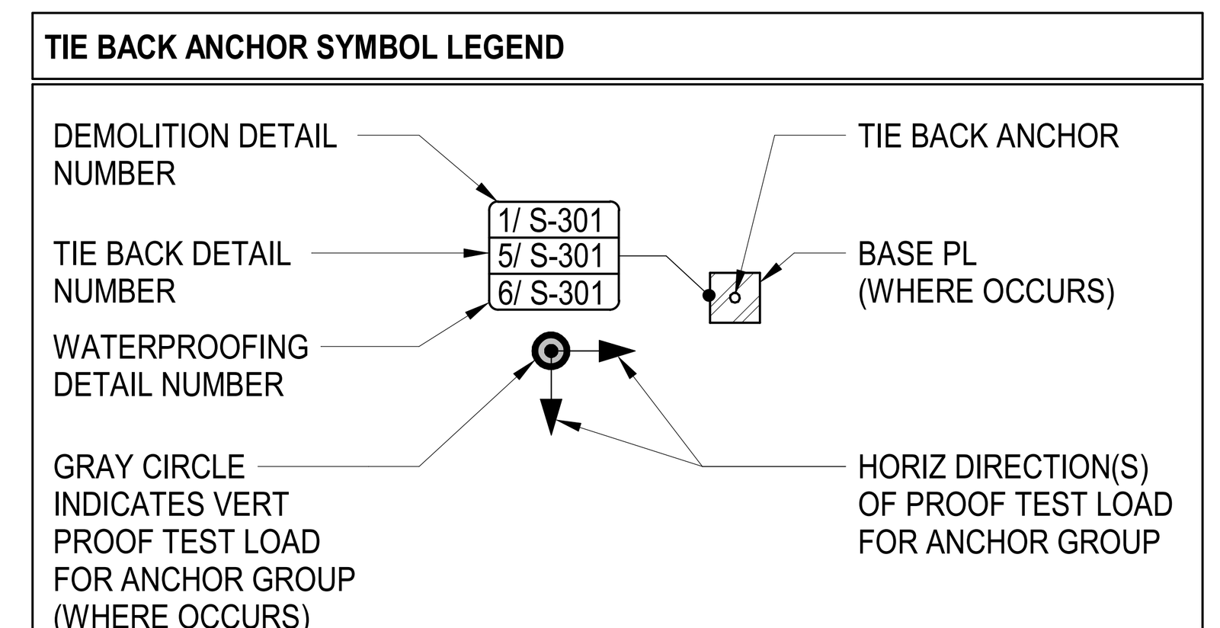
1 PENTHOUSE ROOF PLAN - WEST (ADD ALTERNATE)

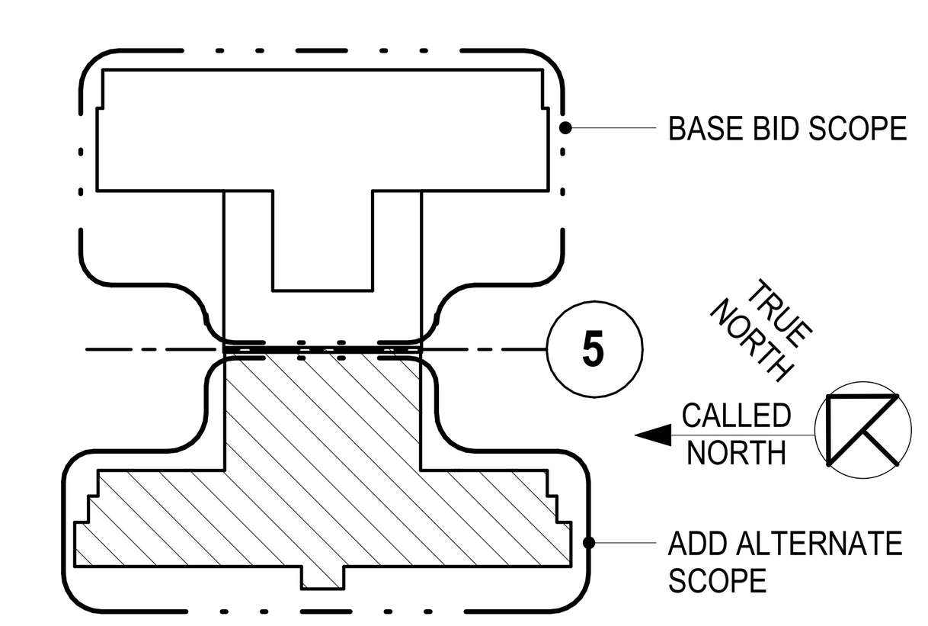
SCALE: 1/8" = 1'-0"

PLAN NOTES:

1. TOP OF STEEL JOIST AT EL = +172'-8 1/2", UON.
   TOP OF STRUCTURAL STEEL AT EL = +172'-6", UON.
2. RD-1 INDICATES EXISTING 1 1/2" 20 GA INTERMEDIATE RIB METAL ROOF DECK. VERIFY IN FIELD.

TIE BACK ANCHOR SYMBOL LEGEND





KEYPLAN

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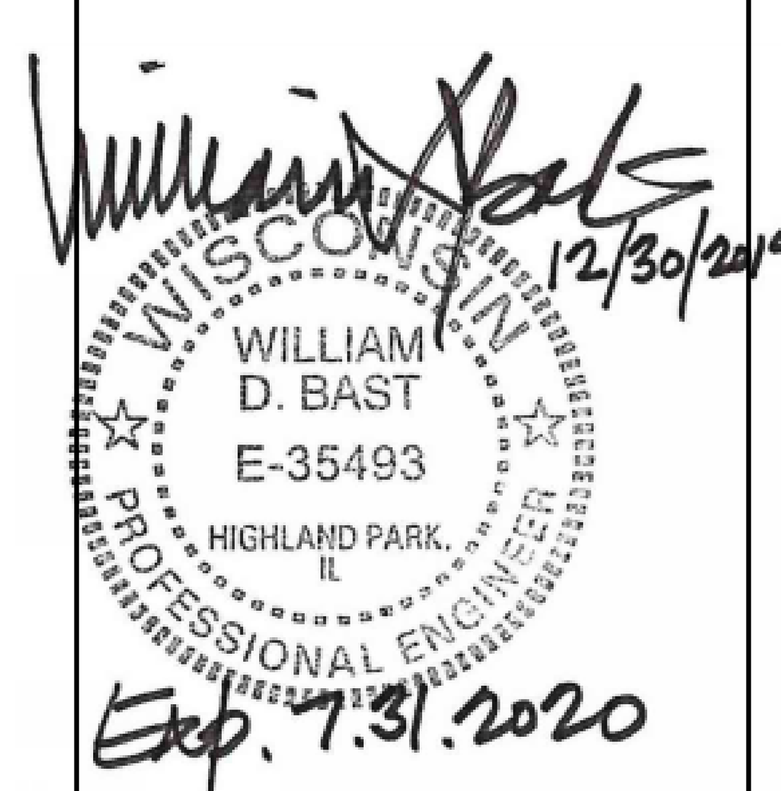

REV.	ISSUE	DATE
1	80% CDs	11/05/19
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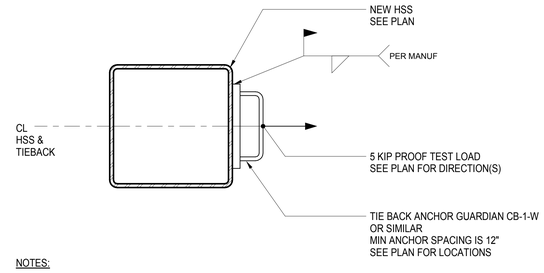
NEW REMOTE DESCENT SYSTEM ANCHORAGE

CITY-COUNTY BUILDING
210 MARTIN LUTHER KING JR BLVD
MADISON, WI 53703

TITLE
PENTHOUSE ROOF PLAN - WEST

PROJECT NO: C19150.00
SCALE: As indicated
DRAWN BY: JRC
CHECKED BY: RTM
DATE: 11/05/2019

SHEET NUMBER
S-192

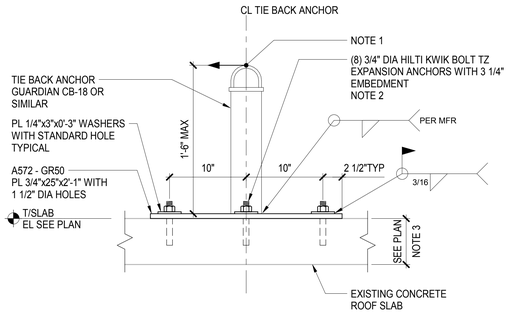


NOTES:

1. TOUCH-UP GALVANIZING ON TUBE WITH ZIN-RICH PRIMER AFTER TIE BACK ANCHORS ARE WELDED.

1 TIE BACK ANCHOR DETAIL - TYPE 1

SCALE: 3" = 1'-0"

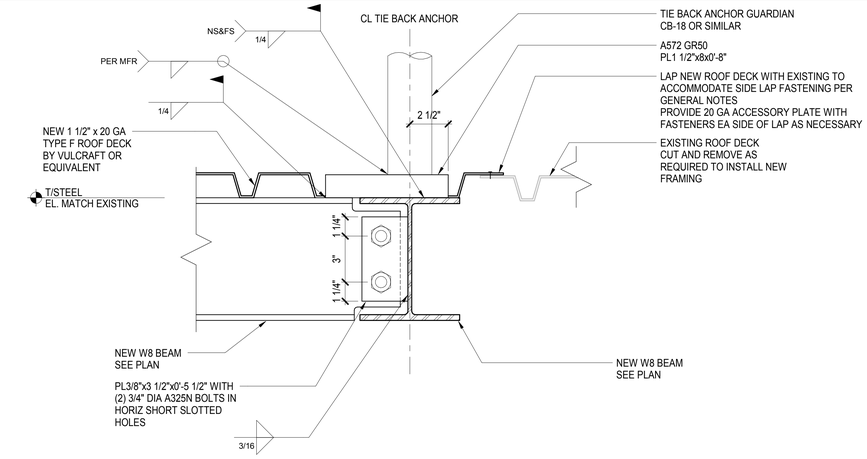


NOTES:

1. TIE BACK ANCHOR BASIS OF DESIGN LOAD IS 5 KIPS APPLIED IN ANY DIRECTION. REFER TO PLAN FOR PROOF TEST LOAD DIRECTION(S).
2. PRIOR TO DRILLING HOLES, LOCATE EXISTING SLAB REINF VIA NON-DESTRUCTIVE MEANS SO AS TO NOT DAMAGE OR CUT EXISTING REINF PER GENERAL NOTE PA-7.
3. CONTRACTOR TO CONFIRM SLAB THICKNESS IN FIELD PRIOR TO FABRICATION & CONSTRUCTION. NOTIFY SER IF SLAB THICKNESS IS LESS THAN SHOWN IN PLAN.

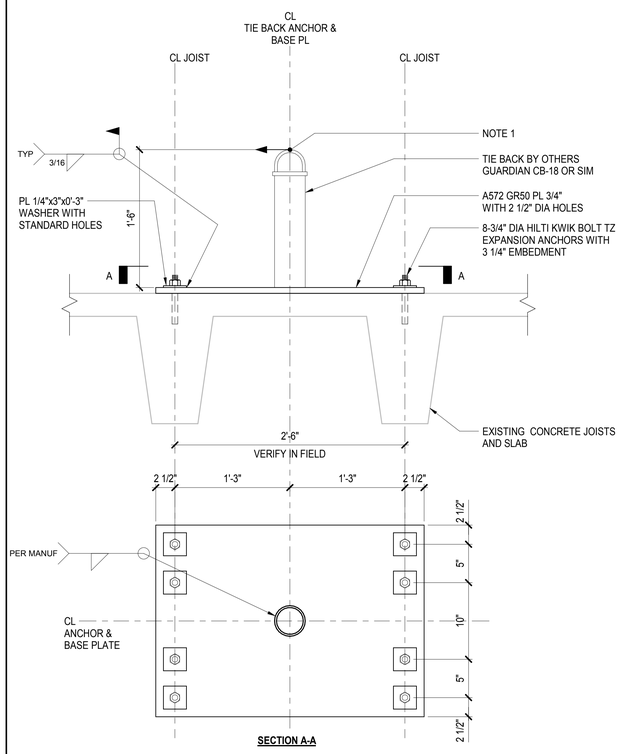
2 TIE BACK ANCHOR DETAIL - TYPE 2

SCALE: 1 1/2" = 1'-0"



3 TIE BACK ANCHOR DETAIL - TYPE 3

SCALE: 3" = 1'-0"



NOTES

1. TIE BACK ANCHOR BASIS OF DESIGN LOAD IS 5 KIPS APPLIED IN ANY DIRECTION. REFER TO PLAN FOR PROOF TEST LOAD DIRECTION(S).
2. PRIOR TO DRILLING HOLES, LOCATE EXISTING SLAB REINF VIA NON-DESTRUCTIVE MEANS SO AS TO NOT DAMAGE OR CUT EXISTING REINF PER GENERAL NOTE PA-7.
3. CONTRACTOR TO FIELD VERIFY EXISTING CONCRETE JOIST LAYOUT. PLAN LOCATION OF ANCHOR ASSEMBLY TO BE SET ACCORDING TO VERIFIED EXISTING JOIST CENTERLINES.

4 TIE BACK ANCHOR DETAIL - TYPE 4

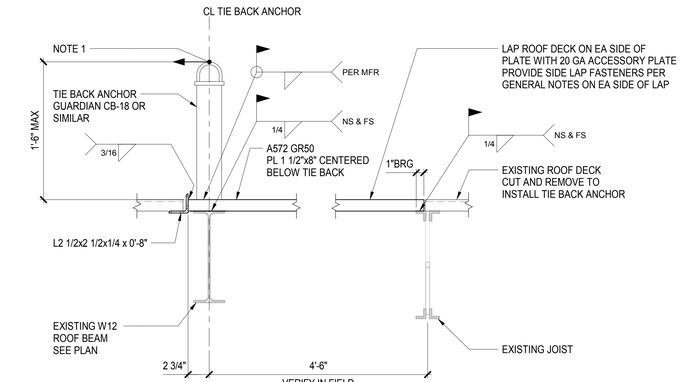
SCALE: 1 1/2" = 1'-0"

NOTES

1. TIE BACK ANCHOR BASIS OF DESIGN LOAD IS 5 KIPS APPLIED IN ANY DIRECTION. REFER TO PLAN FOR PROOF TEST LOAD DIRECTION(S).
2. DO NOT CUT OR DAMAGE EXISTING REINFORCEMENT WHEN REMOVING CONCRETE. ADJUST LOCATION OF TIE BACK ANCHOR AS REQUIRED SO AS TO FIT BETWEEN SLAB TOP REINFORCING BARS.

5 TIE BACK ANCHOR DETAIL - TYPE 5

SCALE: 1 1/2" = 1'-0"

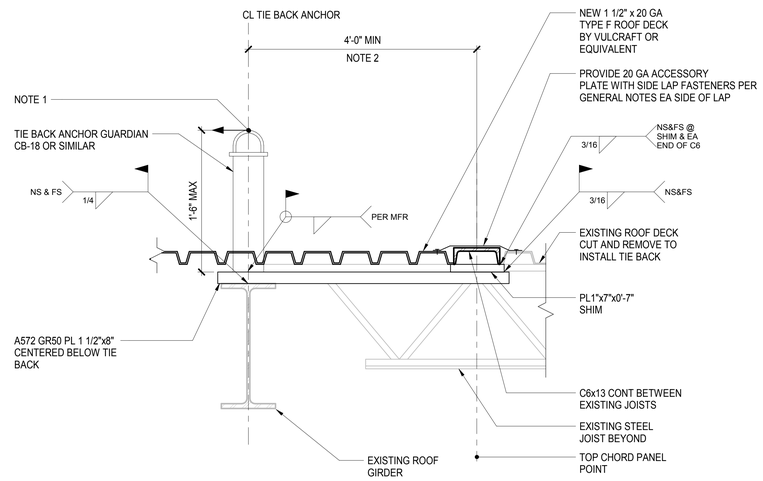


NOTES:

1. TIE BACK ANCHOR BASIS OF DESIGN LOAD IS 5 KIPS APPLIED IN ANY DIRECTION. REFER TO PLAN FOR PROOF TEST LOAD DIRECTION(S).

6 TIE BACK ANCHOR DETAIL - TYPE 6

SCALE: 1 1/2" = 1'-0"

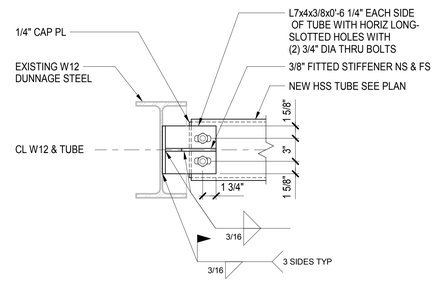


NOTES

1. TIE BACK ANCHOR BASIS OF DESIGN LOAD IS 5 KIPS APPLIED IN ANY DIRECTION. REFER TO PLAN FOR PROOF TEST LOAD DIRECTION(S).
2. ALIGN C6 CHANNEL WITH FIRST JOIST TOP CHORD PANEL POINT 4 FEET OR MORE FROM TIE BACK. WELD TO TOP FLANGE OF EXISTING JOIST WITH 3/16" FILLET WELDS AT EACH TOE.

7 TIE BACK ANCHOR DETAIL - TYPE 7

SCALE: 1 1/2" = 1'-0"



NOTES

1. PROVIDE WEEP HOLES IN BOTTOM OF TUBE AT ENDS AND AT MIDSPAN.
2. TOUCH UP GALVANIZING ON NEW AND EXISTING STEEL WITH ZINC-RICH PRIMER AFTER WELDING HAS BEEN COMPLETED.

8 TUBE CONNECTION DETAIL

SCALE: 1 1/2" = 1'-0"

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HIGHLAND PARK, IL
PROFESSIONAL ENGINEER
12/30/2019
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REV	ISSUE	DATE
1	80% CDs	11/05/19
2	CONSTRUCTION DOCUMENTS	12/30/19

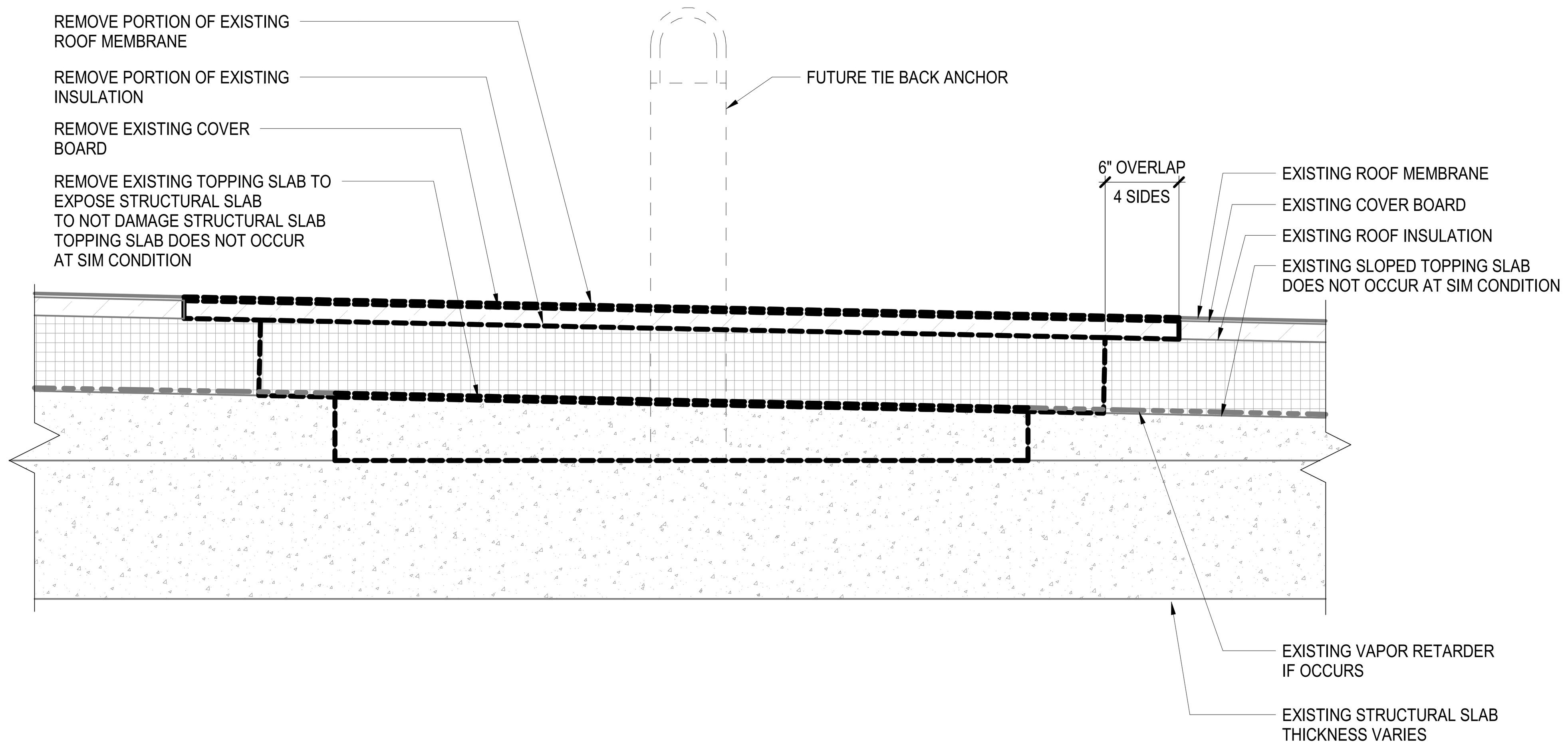
NEW REMOTE DESCENT SYSTEM ANCHORAGE

CITY-COUNTY BUILDING
210 MARTIN LUTHER KING JR BLVD
MADISON, WI 53703

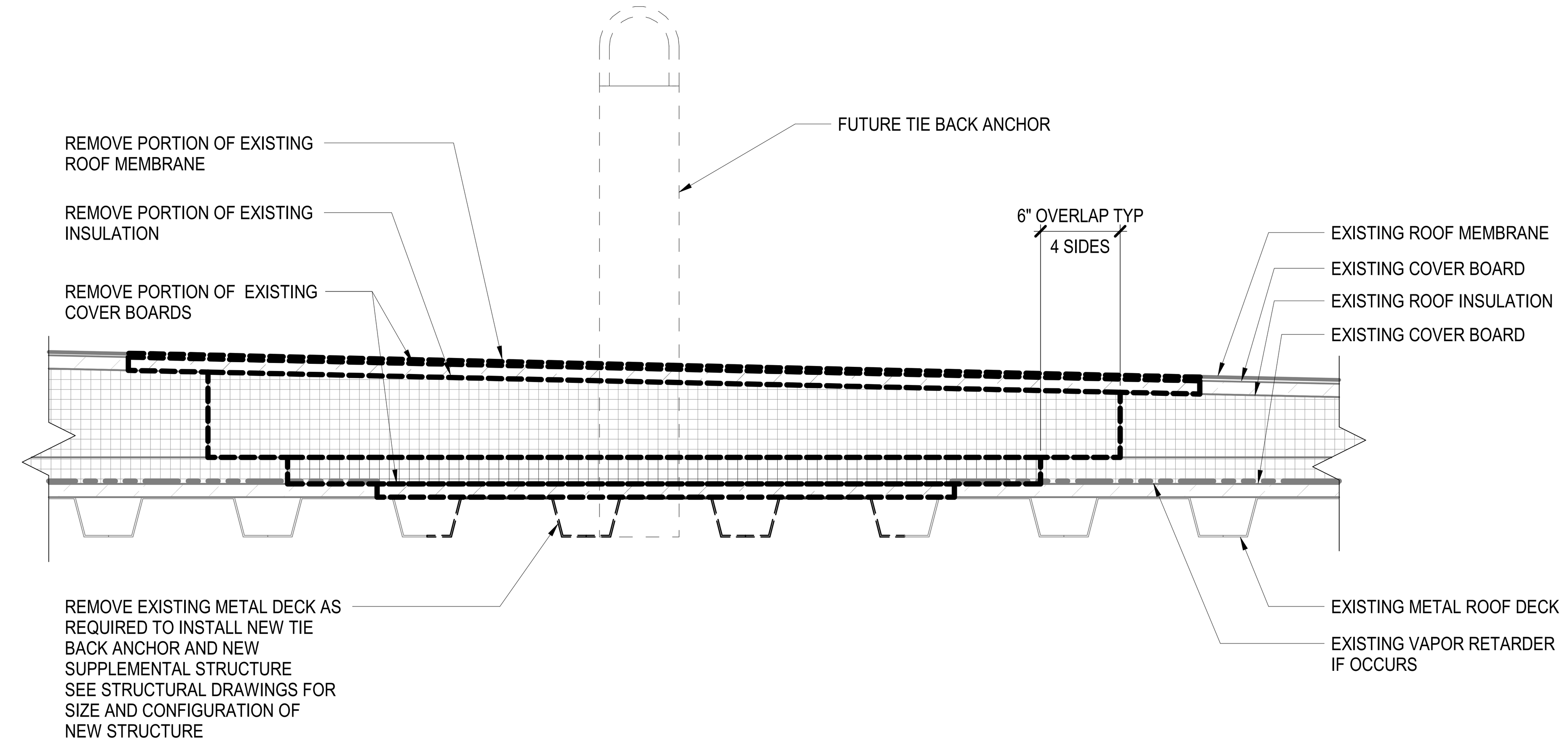
TITLE
STRUCTURAL DETAILS

PROJECT NO:
C19150.00
SCALE:
As indicated
DRAWN BY:
JRC
CHECKED BY:
RTM
DATE:
11/05/2019

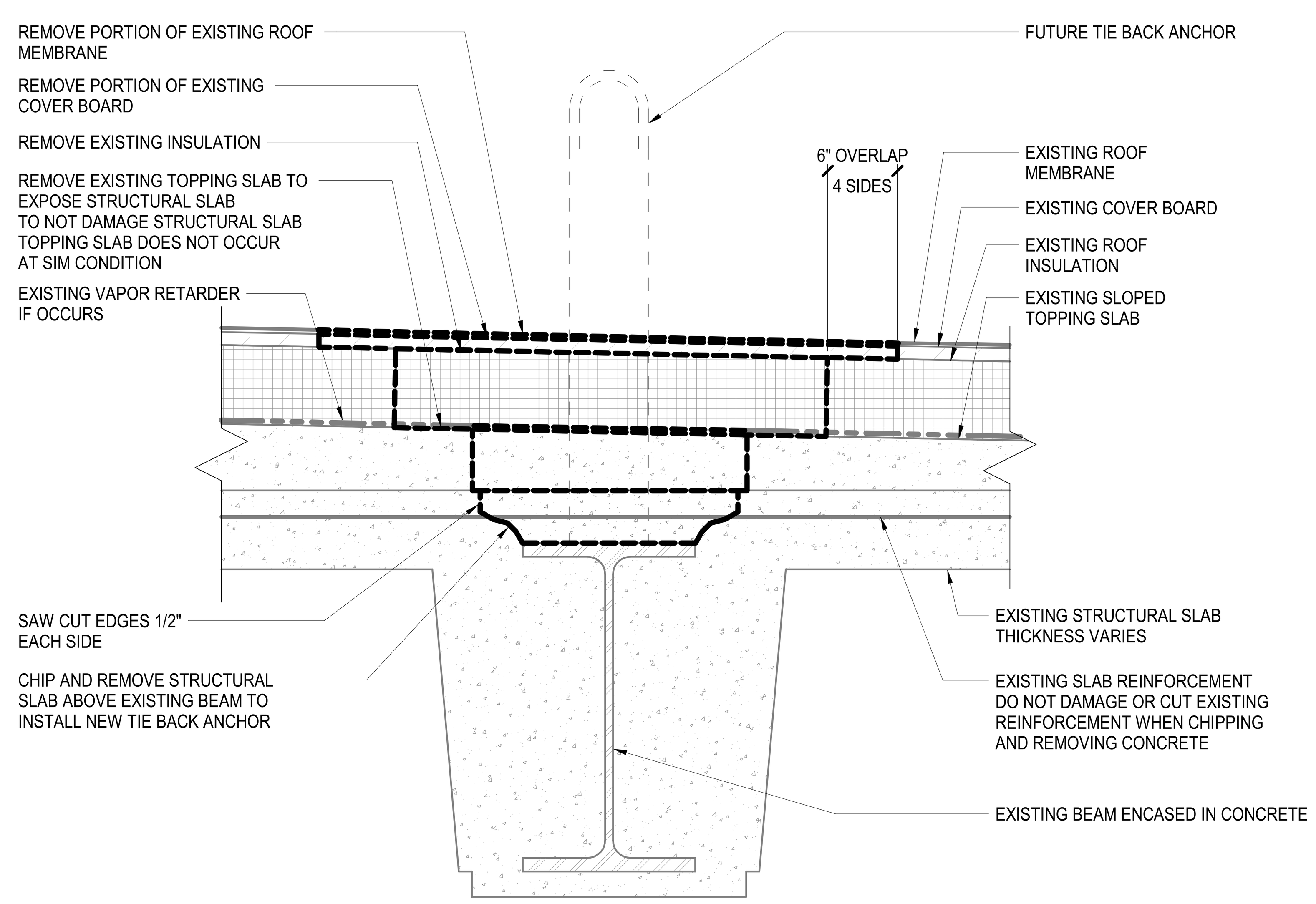
SHEET NUMBER
S-301



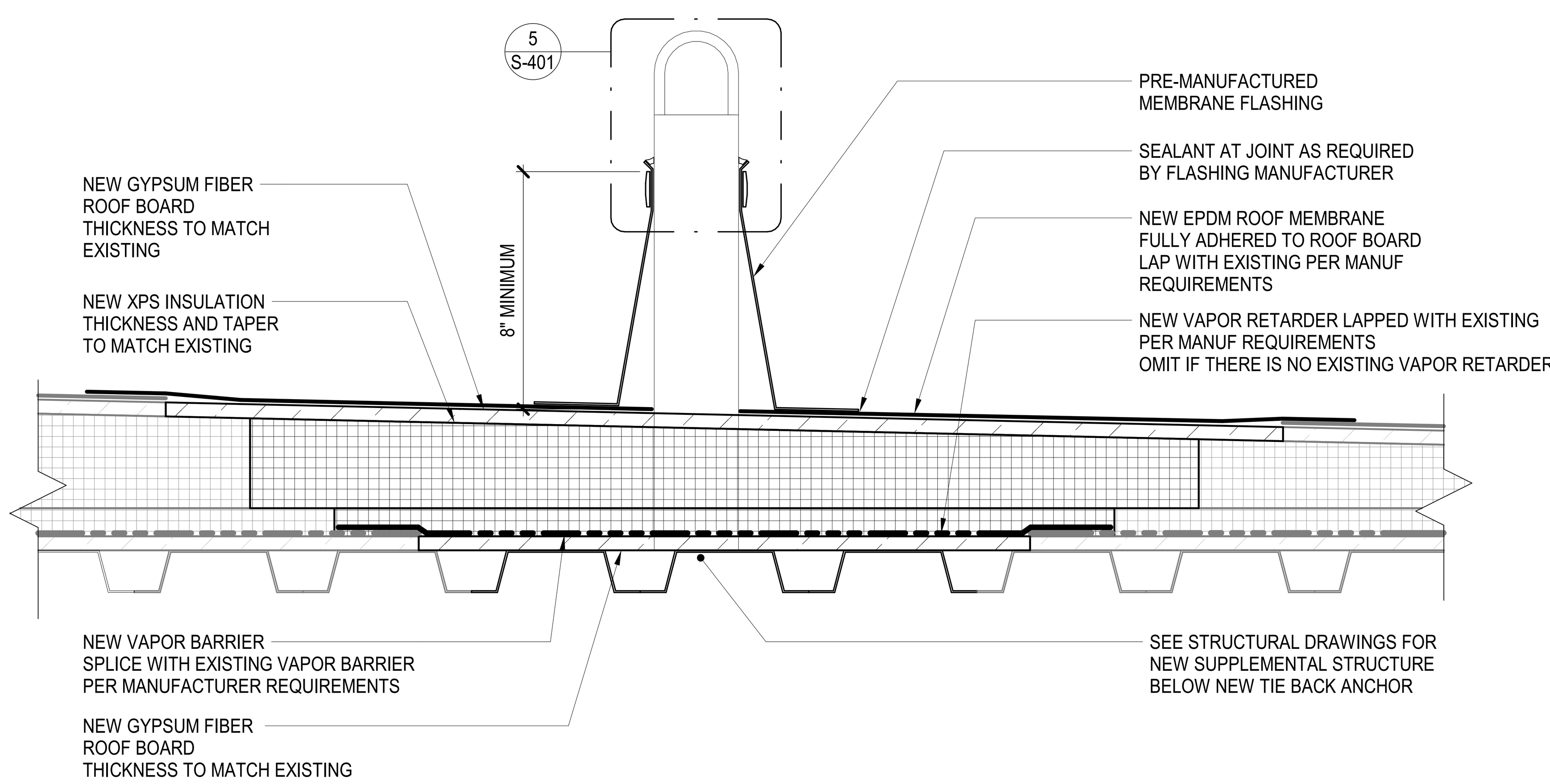
1 DEMOLITION DETAIL
SCALE: 3" = 1'-0"



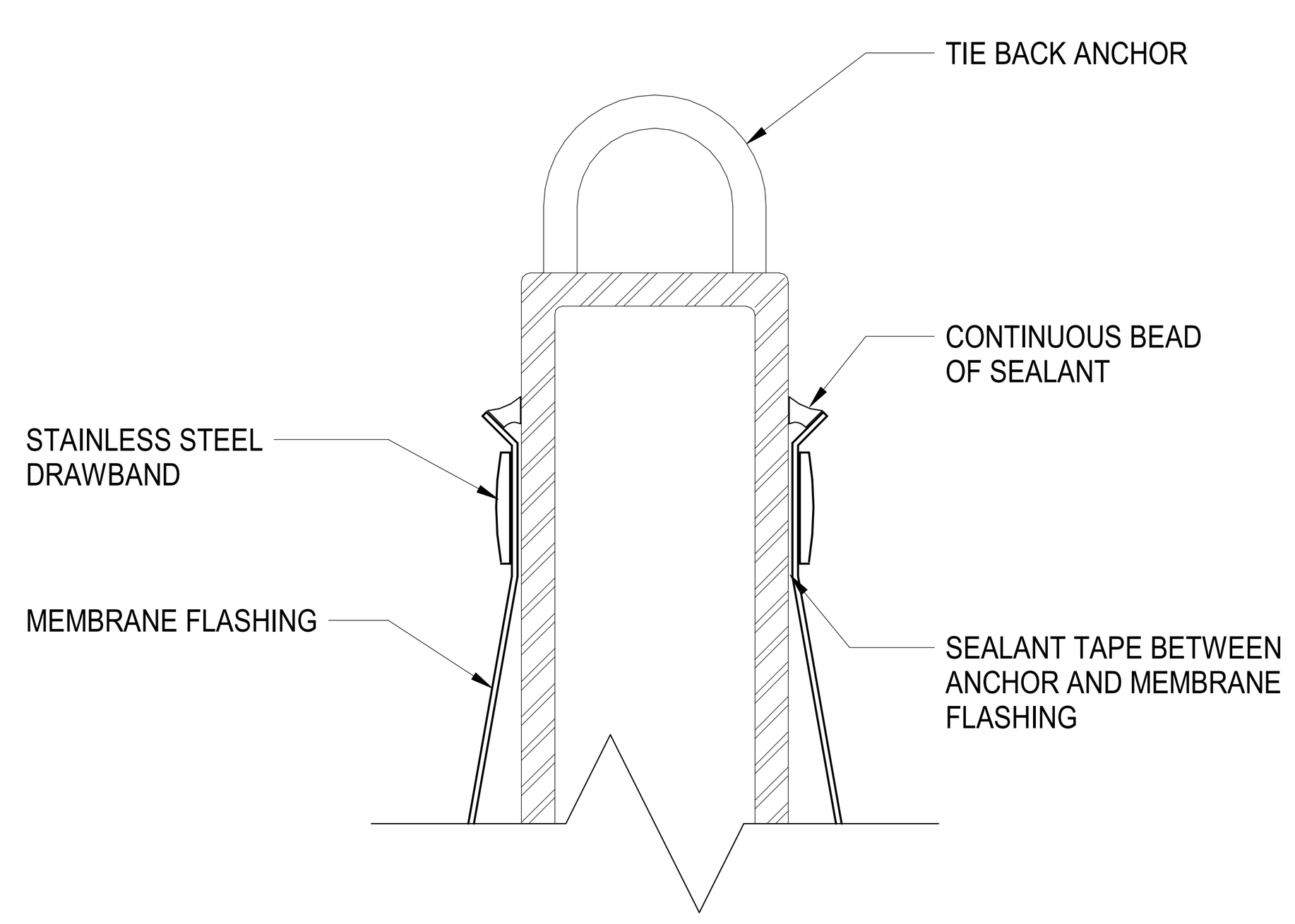
2 DEMOLITION DETAIL
SCALE: 3" = 1'-0"



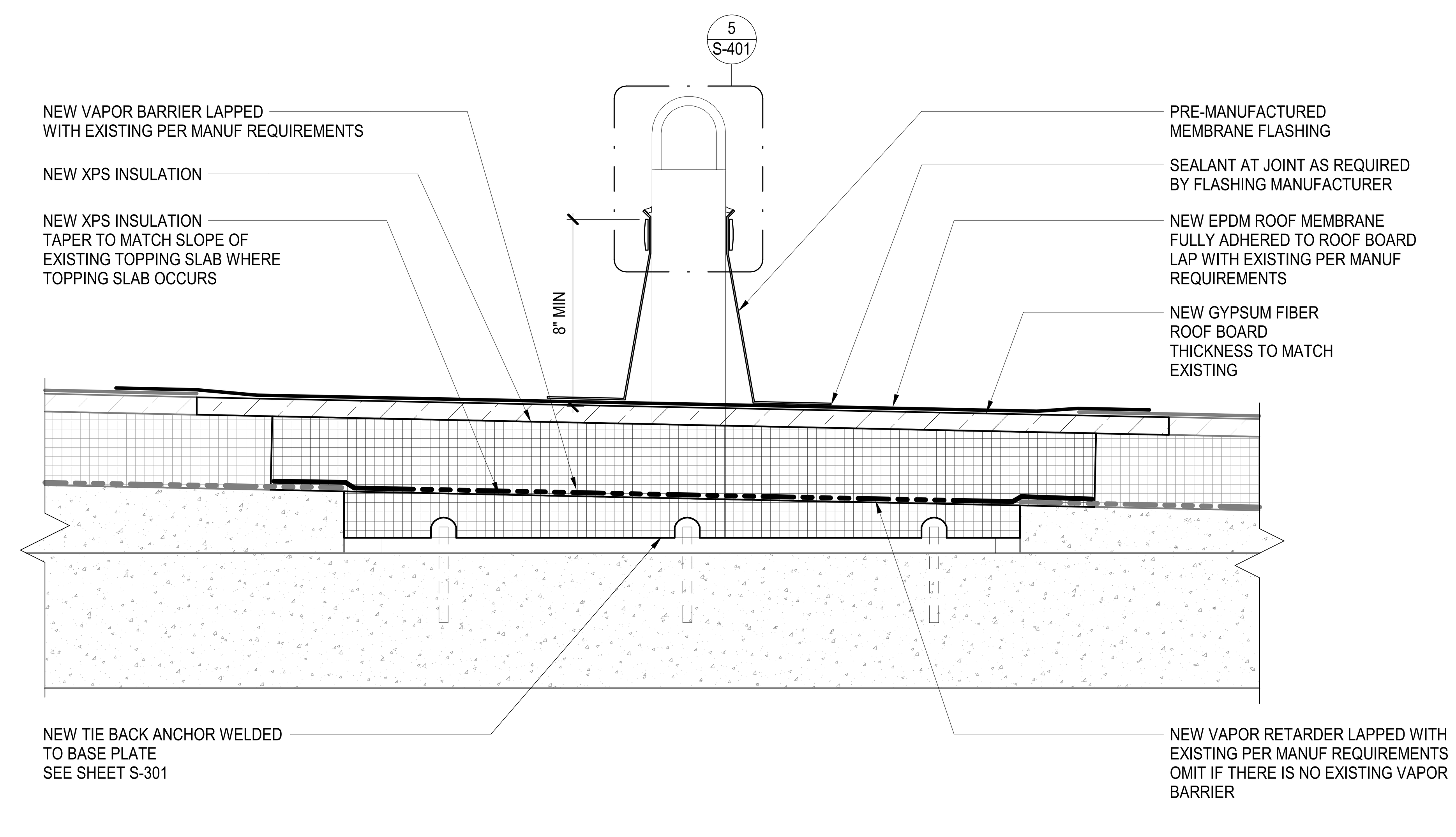
3 DEMOLITION DETAIL
SCALE: 3" = 1'-0"



4 DETAIL OF TIE BACK ANCHOR AT METAL DECK
SCALE: 3" = 1'-0"

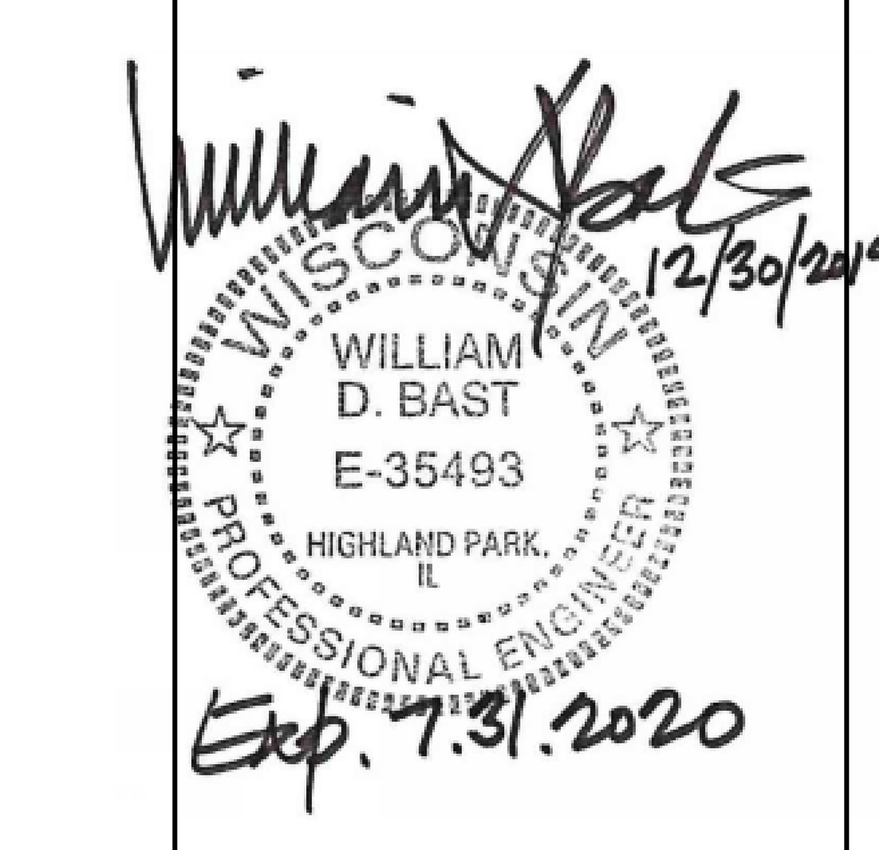


5 DETAIL OF FLASHING AT TOP OF TIE BACK ANCHOR
SCALE: 6" = 1'-0"



6 DETAIL OF TIE BACK ANCHOR AT CONCRETE SLAB
SCALE: 3" = 1'-0"

Thornton Tomasetti
Thornton Tomasetti, Inc.
222 East Erie St, Suite 360
Milwaukee, WI 53202
T 414.875.3370



REV	ISSUE	DATE
1	80% CDs	11/05/19
2	CONSTRUCTION DOCUMENTS	12/30/19

NEW REMOTE DESCENT SYSTEM ANCHORAGE

CITY-COUNTY BUILDING
210 MARTIN LUTHER KING JR BLVD
MADISON, WI 53703

TITLE
DEMOLITION AND WATERPROOFING DETAILS

PROJECT NO: C19150.00
SCALE: AS NOTED
DRAWN BY: JRC
CHECKED BY: RTM
DATE: 11/05/19

SHEET NUMBER
S-401